TECHNICAL DATA NOTEBOOK FOR THE VENTANA CANYON WASH/ESPERERO WASH

LETTER OF MAP REVISION (LOMR)

PIMA COUNTY, ARIZONA

Prepared for:

Pima County Regional Flood Control District 97 East Congress, Third Floor Tucson, Arizona 85701

By:

JE Fuller / Hydrology & Geomorphology, Inc. 40 East Helen Street Tucson, Arizona 85705 520-623-3112

July 8, 2009

<u>Final</u> September 13, 2010



TDN – VENTANA CANYON WASH/ESPERERO WASH LOMR

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TDN – VENTANA CANYON WASH/ESPERERO WASH LOMR

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I. INTRODUCTION

The Pima County Regional Flood Control District (RFCD) contracted with JE Fuller/Hydrology and Geomorphology, Inc. (JEF) to prepare this request for a Letter of Map Revision (LOMR) for the Ventana Canyon Wash and the Esperero Wash (see Figures 1 and 2). The Pima County contract number is PO 09019014, and the project manager is Terry Hendricks, CFM, Chief Hydrologist. A copy of the scope of work is included in Appendix B.

This LOMR will address changes to the regulatory flood plain as currently mapped on Panels 1655, 1663, and 1665 (Reference 1). This Technical Data Notebook (TDN), which follows the outline specified in Arizona Department of Water Resources, State Standard 1-97, contains all the information required in support of this request, including the applicable Federal Emergency Management Agency (FEMA) forms.

The request is based on a more detailed hydrologic analysis of the contributing watersheds and on updated topographic mapping. It consolidates the numerous Letters of Map Change (LOMC) that have been issued since the current mapping became effective and includes revisions to areas affected by private developments. In addition, the effect of one newly constructed culvert and enlarged bridge are accounted for with the revised mapping.

The project reach for the Ventana Canyon Wash begins at the Tanque Verde Creek confluence, which is the downstream limit of the effective mapping, and extends approximately 6.6 miles upstream to the boundary of the Coronado National Forest. The majority of the project reach was previously mapped by detailed methods using HEC-2 (Reference 2) and includes a combination of Zone AE and shaded Zone X designations. However, approximately 1000 feet was only mapped by approximate methods (i.e., the effective Zone A areas), and approximately 1000 feet was not previously mapped. The revised mapping of the entire project reach is by detailed methods. The modeling was performed using HEC-RAS (Reference 3).

The project reach for the Esperero Wash begins at the Ventana Canyon Wash confluence and extends approximately 1.8 miles. The majority of the project reach was previously mapped by detailed methods (HEC-2) and includes a combination of Zone AE and shaded Zone X designations. However, approximately 1900 feet was only mapped by approximate methods (i.e., the effective Zone A areas), and approximately 800 feet was not previously mapped. The revised mapping of the entire project reach is by detailed methods (HEC-RAS).

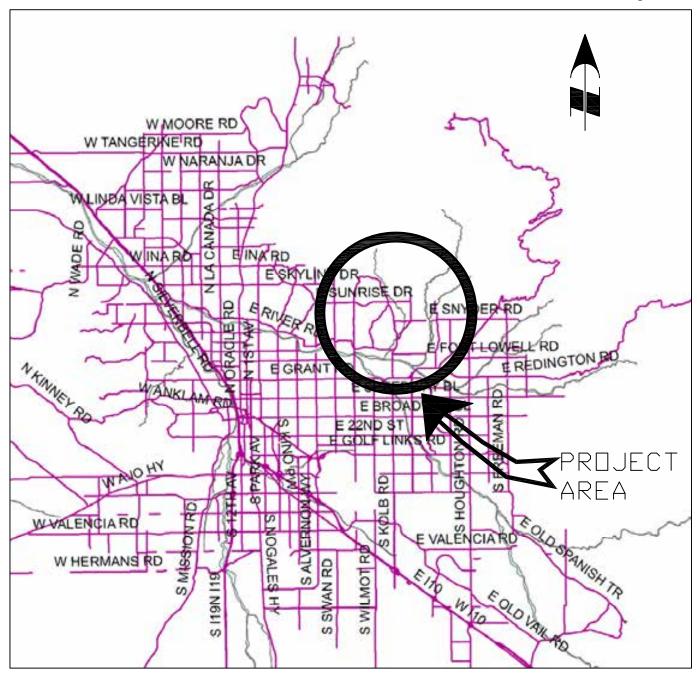
The RFCD provided JEF with a base model that was complied by Tetra Tech, Inc. in 2002 under contract with the RFCD (Reference 5). However, Tetra Tech's base model was limited to the 100-year return interval. A copy of the Tetra Tech's summary report is provided in PDF format in Appendix D. As part of this LOMR request, per the Pima County's current hydrologic modeling criteria, JEF updated the base model to incorporate (1) the new NOAA Atlas Volume 14 data; (2) the 3-hour, upper 90% confidence interval precipitation values from NOAA 14; (3) the Hydro-40 aerial reduction factors, and (4) the SCS Type II distribution. In addition, JEF expanded the modeling effort to included the 10-, 50-, 100-, and 500-year return intervals. A total of ten point-precipitation-value data sets were obtained from the NOAA 14 web site based

on the longitude and latitude of the centroid of each the ten major sub-basins. However, since the range of individual values was narrow, an average value was selected for each return interval. An areal reduction factor was then applied to define a basin average value for each of the ten major sub-basins. The results of the HEC-1 analysis are summarized in Table 1.1. In addition, Table 1.1 provides a comparison between the effective discharges and the revised discharges. The revised HEC-1 models are provided in Appendix D. A printout of the watershed map is also provided in Appendix D.

As previously noted, the hydraulic analysis was conducted using HEC-RAS. Using the results of the HEC-1 modeling, interpolated discharge values were defined at key locations in the HEC-RAS model in an attempt to maintain the gradually-varied flow assumption while accounting for the variation in peak discharges in the downstream direction. A combination of as-built plans and field survey (performed by Stantec Consulting, Inc.) was used to define the geometry of all bridge openings and/or culvert crossing. The n-values were determined from an extensive field investigation. The results of that investigation, with supporting documentation (field photographs), was summarized in a field reconnaissance report (Reference 8). A copy of the field reconnaissance report prepared by JEF and the field notes from Stantec's survey are provided in Appendix C. The revised 500-year and 100-year floodplain and floodway boundaries are shown on the attached work maps.

The topographic information used in conjunction with the re-mapping was based on the North American Vertical Datum of 1988 (NAVD 88). The effective mapping was based on the National Geodetic Vertical Datum of 1929 (NGVD 29). Therefore, both elevations are shown on the work maps. A tabular listing of water-surface elevations for both datums is also provided in Appendix E. The difference in elevations is based on a single conversion factor – 2.26 feet (i.e., the NAVD 88 elevations are approximately 2.26 feet higher than the NGVD 29 elevations).

The only ties to the effective mapping occur at the Tanque Verde Creek confluence. The effective base water-surface elevation in the Tanque Verde Creek at the confluence is approximately 2463.0 (NGVD 29) or 2465.25 (NAVD 88). Since the computed water-surface elevation for the Ventana Canyon Wash at the confluence is 2467.4 (NGVD or 2469.7 (NAVD), which is based on critical depth, there is no hydraulic tie between the Tanque Verde Creek and the Ventana Canyon Wash. Therefore, graphic ties were made between the revised floodplain/floodway boundaries for the Ventana Canyon Wash and those associated with the Tanque Verde Creek.



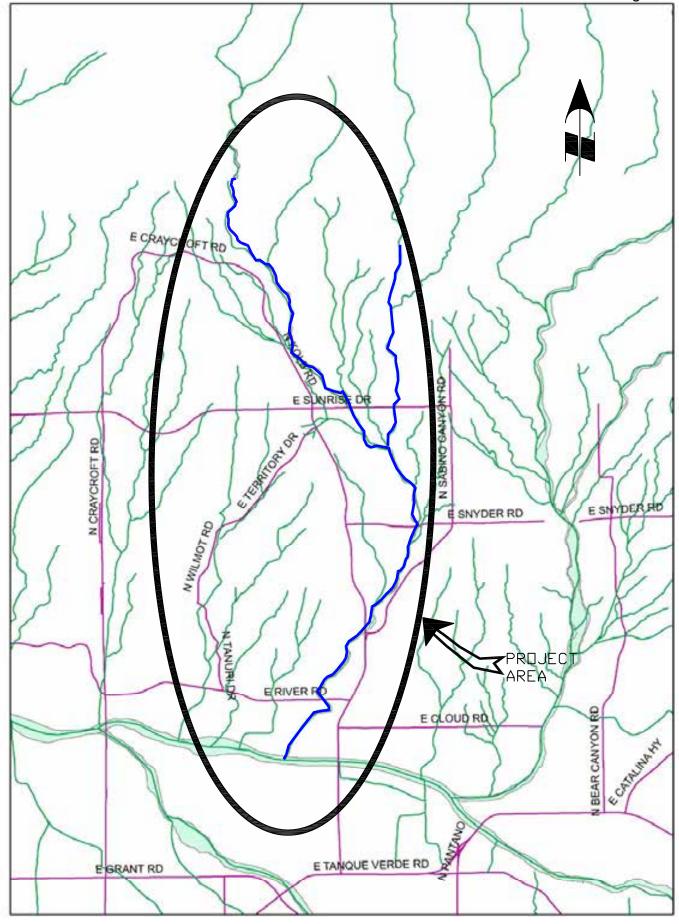


Figure 2

Table 1.1 Summary of Applied Discharges for Esperero Wash and Ventana Canyon Wash LOMR (with a comparison between the effective and revised discharges)

Flooding	Concentraton Drainage	Drainage	Ö	0	ď	Q_{50}	δ	00	ඊ	λ_{500}
Source	Point	Area	effective	revised	effective	revised	effective	revised	effective	revised
		(sq mi)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)
	Node 4	5.9	4239	5121	8789	8907	11037	10762	19000	15953
Esperero Wash	Node 5	6.11	3350	4333	7319	2007	9116	9170	16000	13663
	Node 6	6.19	2947	4246	6795	6949	8440	8888	14400	13574
	Node 8	3.85	3304	5179	6621	8813	9882	10596	13250	14864
Ventana	Node 10	6.98	4172	5378	8684	9448	10770	12044	19500	17805
Canyon	Node 11	7.94	4140	5271	8888	9151	11082	11484	18500	17544
Wash	Node 6 +11	14.14	4952	8122	11451	14053	14775	17753	27000	27253
	Node 15	16.64	3217	5066	1	9030	9371	11527	17000	18238

II. ADWR/FEMA FORMS

2.1 Study Documentation Abstract (LOMR)

Subsection	Information Requested	Response
2.1.1	Date Study Accepted:	Accepted: April 27, 2010 Effective: September 13, 2010.
2.1.2	Study Contractor:	JE Fuller/Hydrology & Geomorphology, Inc.
	Contact:	Robert L. Shand, P.E., Project Manager
	Address:	40 E. Helen Street
		Tucson, Arizona 85705
	Phone:	520-623-3112
	Internal Reference Number:	PCRFCD-Ventana_Esperero_LOMR
2.1.3	FEMA Technical Review Contractor:	Baker AECOM
	Contact:	Mounir Boudjemaa, M.S.
	Address:	c/o LOMC Clearinghouse
		6730 Santa Barbara Court
		Elkridge, MD 21075
	Phone:	703-317-6295
	Internal Reference Number:	Case No. 09-09-2406P
2.1.4	FEMA Regional Reviewer:	Director, Mitigation Division, Dept. of Homeland Security's FEMA, Oakland, CA
	Phone:	510-627-7175
2.1.5	State Technical Reviewer:	Brian Cosson, CFM.
	Phone:	602-771-8657
2.1.6	Local Technical Reviewer:	Terry Hendricks, CFM
		Chief Hydrologist
	Phone:	520-740-6350
2.1.7	Reach Descriptions (approximate):	(1) 6.6 miles of the Ventana Canyon Wash beginning a the Tanque Verde Creek confluence;
		(2) 1.8 miles of the Esperero Wash beginning at the Ventana Canyon Wash confluence.

Subsection	Information Requested	Response
2.1.8	USGS Quad Sheets:	Sabino Canyon., 7.5' quadrangle, 1992
	Digital Mapping provided by Pima Association of Governments	1998 and 2000 topo/photo coverage of study area
2.1.9	Unique Conditions and Problems:	None
2.1.10	Coordination of Peak Discharges:	Pima County concurs with the application of the revised peak discharges.

2.2 FEMA Forms

FEMA MT-2 Forms 1, 2 and 3 are included as attachments to this section. Each form includes a supplemental information sheet that provides information that could not be placed within the form structure. The form's section and item number is referenced on this sheet. Normally, Sections 3 through 7 of the standard TDN provides overflow information that cannot be placed within the FEMA form structure. If the information requested in Sections 3 through 7 is already provided on the supplemental information sheet, it will be referenced accordingly. In addition, some of the information requested in Sections 3 through 7 may be provided in one of the appendices. If that is the case, the location where the information can be found will be referenced accordingly.

U.S. DEPARTMENT OF HOMELAND SECURITY - FEDERAL EMERGENCY MANAGEMENT AGENCY **OVERVIEW & CONCURRENCE FORM**

O.M.B No. 1660-0016 Expires: 12/31/2010

PAPERWORK BURDEN DISCLOSURE NOTICE

Public reporting burden for this form is estimated to average 1 hour per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing, reviewing, and submitting the form. You are not required to respond to this collection of information unless a valid OMB control number appears in the upper right corner of this form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden to: Information Collections Management, U.S. Department of Homeland Security, Federal Emergency Management Agency, 500 C Street, SW, Washington DC 20472, Paperwork Reduction Project (1660-0016). Submission of the form is required to obtain or retain benefits under the National Flood Insurance Program. Please do not send your completed survey to the above address.

A. REQUESTED RESPONSE FROM DHS-FEMA

This request is for a (chec	k one):
☐ CLOMR:	A letter from DHS-FEMA commenting on whether a proposed project, if built as proposed, would justify a map revision, or proposed hydrology changes (See 44 CFR Ch. 1, Parts 60, 65 & 72).
	A letter from DHS-FEMA officially revising the current NFIP map to show the changes to floodplains, regulatory floodway or flood elevations. (See 44 CFR Ch. 1, Parts 60, 65 & 72)
	B OVERVIEW

B. OVERVIEW								
1. The NFIP map p	anel(s) affected	for all impacted communities	is (are):					
Community No.	Community Na	me			State	Map No.	Panel No.	Effective Date
Ex: 480301	City of Katy				TX	480301	0005D	02/08/83
480287	Harris County				TX	48201C	0220G	09/28/90
040073		rizona and Incorporated Area			AZ	04019C	1655K	02/08/99
040073	Pima County A	rizona and Incorporated Area	as		AZ	04019C	1663K	04/22/04
2. a. Flooding Sou	ırce: Ventana Ca	anyon Wash, Esperero Wash						
b. Types of Floo	ding: 🛛 Riverir	ne 🗌 Coastal 🔲 Sha	allow Flo	oding (e.g., Zo	nes AO ar	nd AH)		
	☐ Alluvial	fan Lakes Oth	er (Atta	ch Description)				
3. Project Name/Id	entifier: Ventana	Canyon Wash/Esperero Was	sh LOMF	3				
4. FEMA zone designations affected: A, AE, and X (shaded) (choices: A, AH, AO, A1-A30, A99, AE, AR, V, V1-V30, VE, B, C, D, X)								
5. Basis for Reques	st and Type of R	evision:						
a. The basis f	or this revision re	equest is (check all that apply))					
□ Physica	Change		Data	□ Regulatory	y Floodwa	/ Revision	☐ Base Map (Changes
☐ Coastal	Analysis		lydraulic Analysis		☐ Hydrologic Analysis		☐ Corrections	
☐ Weir-Da	m Changes	☐ Levee Certification		☐ Alluvial Fa	an Analysis	5	☐ Natural Cha	anges
New Top New Top	ographic Data	☐ Other (Attach Descriptio	n)					
Note: A ph	otograph and na	rrative description of the area	of conc	ern is not requi	red, but is	very helpful d	uring review.	
b. The area of revis	sion encompasse	es the following structures (ch	eck all tl	hat apply)				
Structures:		☐ Channelization	☐ Lev	/ee/Floodwall	\boxtimes	Bridge/Culvert		
		☐ Dam	☐ Fill			Other (Attach	Description)	
						•	• •	

C. REVIEW FEE

Has the review fee for the appropriate request category b	een included?		☐ Yes	Fee am	ount: \$
			⊠ No, Attach	Explanation	
Please see the DHS-FEMA Web site at http://www.fem	na.gov/plan/preve	ent/fhm/frm_fee	s.shtm for Fee	Amounts a	nd Exemptions.
	D. SIGN	IATURE			
All documents submitted in support of this request are confine or imprisonment under Title 18 of the United States C			understand that	any false sta	atement may be punishable by
Name: Robert L. Shand		Company: JE	Fuller/Hydrology	y & Geomorp	hology, Inc
Mailing Address: 40 E. Helen Street		Daytime Telepi	none No.: 520-	523-3112	Fax No.: 520-623-3130
Tucson, AZ 85705		E-Mail Address	: rob_shand@j	efuller.com	
Signature of Requester (required):	Holla	Que	Date: July 0	8, 2009	
(LOMR) or conditional LOMR request. Based upon the confidence of the community floodplain management requirements, in Federal, State, and local permits have been, or in the case any existing or proposed structures to be removed from the have available upon request by FEMA, all analyses and decommunity Official's Name and Title: Suzanne Shields, F	ncluding the require e of a conditional LO ne SFHA are or will ocumentation used	ment that no fill to OMR, will be obta be reasonably sa to make this det	e placed in the sined. In addition of from flooding ermination.	regulatory flo	podway, and that all necessary determined that the land and in 44CFR 65.2(c), and that we
Regional Flood Control District		30.500		September 1	
Mailing Address: 97 East Congress Street, 3rd Floor		Daytime Telepi	none No.: 520-	243-1800	Fax No.: 520-243-1821
Tucson, AZ 85701		E-Mail Address	: suzanne,shie	lds@rfcd.pin	na.gov
Community Official's Signature (required):	vinne S	helds	Date:	1/91	09
CERTIFICATION BY REGISTE This certification is to be signed and sealed by a licensed elevation information data, hydrologic and hydraulic analy described in the MT-2 Forms Instructions. All documents any false statement may be punishable by fine or imprison	land surveyor, regis sis, and any other s submitted in suppo	stered profession supporting inform ort of this request	al engineer, or ation as per NF are correct to the	architect auth IP regulation se best of my	norized by law to certify s paragraph 65.2(b) and as
Certifier's Name: Robert L. Shand		License No.: 2	4026	Expi	ration Date: 06/30/2010
Company Name: JE Fuller/Hydrology & Geomorphology,	Inc	Telephone No.	520-623-3112	Fax	No.: 520-623-3130
Signature:				Date	: 09/24/2008
Ensure the forms that are appropriate to your revision	request are inclu	ded in your sub	mittal.	-	
Form Name and (Number)	Required if				
☐ Riverine Hydrology and Hydraulics Form (Form 2)	New or revised dis	scharges or water	-surface elevat	ions	
⊠ Riverine Structures Form (Form 3)	Channel is modifie addition/revision o				
☐ Coastal Analysis Form (Form 4)	New or revised co	astal elevations			
☐ Coastal Structures Form (Form 5)	Addition/revision of	of coastal structur	e		Seal (Optional)
☐ Alluvial Fan Flooding Form (Form 6)	Flood control mea	sures on alluvial	fans	L	

MT-2, Form 1 – Overview and Concurrence (Supplemental Information)

Note: This supplemental information applies either in whole or in part to all flooding sources.

■ Section B – OVERVIEW

• Item 1, Panels Affected/Effective Date – Panels 1655K (effective 02/08/99), 1663K (effective 02/08/99, LOMR 04/22/04), and 1665 (effective 02/08/33, LOMR 07/24/00).

■ Section C – REVIEW FEE

• This map change request is based on more detailed hydrologic and hydraulic analyses and is intended to improve upon the information shown on the effective map and within the effective FIS. In addition, the request provides detailed mapping to replace areas that were previously mapped by approximate methods.

U.S. DEPARTMENT OF HOMELAND SECURITY - FEDERAL EMERGENCY MANAGEMENT AGENCY

RIVERINE HYDROLOGY & HYDRAULICS FORM

O.M.B No. 1660-0016 Expires: 12/31/2010

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	Flooding Source: Ventana Canyon Wash Note: Fill out one form for each flooding source studied						
			A. HYI	DROLOGY			
1.	Reason for New Hydrologic A	nalysis (check	all that apply)				
	☐ Not revised (skip to section	on B)	☐ No existing analys	sis			
			☐ Proposed Condition	ons (CLOMR)	☐ Changed physic	cal condition of watershed	
2.	Comparison of Representativ	e 1%-Annual-C	hance Discharges				
	Location	Draii	nage Area (Sq. Mi.)	Effective/	FIS (cfs)	Revised (cfs)	
up	upstream of Resort Drive 3.85 7836 10596						
downstream of Ventana W. 14.14 14775 17753							
at	Tanque Verde Wash	16.64		9371	11	1527	
3.	Methodology for New Hydrolo	ogic Analysis (d	heck all that apply)				
	Statistical Analysis of Gar Regional Regression Equ		☑ Precipitation/Runo☐ Other (please atta				
	Please enclose all relevant m the new analysis.	odels in digital	format, maps, computati	ons (including compu	tation of parameters) a	and documentation to support	
4.	Review/Approval of Analysis						
	If your community requires a	regional, state,	or federal agency to revi	ew the hydrologic ana	alysis, please attach e	vidence of approval/review.	
5.	Impacts of Sediment Transpo	ort on Hydrology	,				
	Was sediment transport cor your explanation for why sed			hen fill out Section F (Sediment Transport) o	of Form 3. If No, then attach	

B. HYDRAULICS

1.	Reach to be Revised				
		Description	Cross Section		ce Elevations (ft.)
	Downstream Limit	confluence with Tanque Verde Crk	RS 2, A (effective)	Effective 2473.1 NGVD	Proposed/Revised 2472.1 NGVD
	Upstream Limit	of detailed study	RS 81	n/a	3220.6 NGVD
2.	Hydraulic Method/Model Used HEC-RAS Version 4				

	B. HYDRAULICS (CONTINUED)							
3.	Pre-Submittal Review of Hydraulic Models							
	DHS-FEMA has developed two review progrespectively. These review programs may NFIP requirements, and that the data are cidentify areas of potential error or concerdownloaded from http://www.fema.gov/plan/CHECK-2 and CHECK-RAS. Review of you	help verify that the hycomparable with the a rn. These tools do /prevent/fhm/frm_soft.s	nydraulic estimates and assumptions and limitate not replace enginee shtm. We recommend	I assumptions in the montrions of HEC-2/HEC-RAPTING judgment. CHE I that you review your H	odel data are in acco AS. CHECK-2 and C ECK-2 and CHECK-F IEC-2 and HEC-RAS	ordance with CHECK-RAS RAS can be models with		
4.	Models Submitted	<u>Natura</u>	al Run	Floodwa	ay Run	<u>Datum</u>		
NAV	Duplicate Effective Model* Corrected Effective Model* Existing or Pre-Project Conditions Model Revised or Post-Project Conditions Model VD	File Name: File Name: File Name: File Name: Ventana	Plan Name: Plan Name: Plan Name: a Plan Name:	File Name:	Plan Name: Plan Name: Plan Name: a Plan Name:	<u> </u>		
1	Other - (attach description)	File Name:	Plan Name:	File Name:	Plan Name:			
* Fo	or details, refer to the corresponding section of	of the instructions.						
<u> </u>		□ Digital Models	s Submitted? (Required	i)				
		C. MAPPING	G REQUIREMENTS	-	=			
prop flood indid requ	pertified topographic map must be submitted posed conditions 1%-annual-chance floodplay deplains and regulatory floodway (for detailed icated; stream, road, and other alignments uester's property; certification of a registered the referenced vertical datum (NGVD, NAVD)	lain (for approximate Zone AE, AO, and AF s (e.g., dams, levees I professional engineel	Zone A revisions) or H revisions); location and s, etc.); current commits.	the boundaries of the and alignment of all cross aunity easements and	1%- and 0.2%-ann s sections with station boundaries; boundaries	nual-chance ning control ries of the		
į		□ Digital Mapping	(GIS/CADD) Data Subr	mitted				
mus to s	te that the boundaries of the existing or proposest tie-in with the effective floodplain and reguleshow the boundaries of the revised 1%- and ective 1%- and 0.2%-annual-chance floodplain	ilatory floodway bound nd 0.2%-annual-chanc n and regulatory floodw	daries. Please attach a ce floodplains and regu way at the upstream and	a copy of the effective ulatory floodway that tie d downstream limits of the	FIRM and/or FBFM, e-in with the bounda	, annotated		
<u> </u>								
	D. COMMON REGULATORY REQUIREMENTS*							
1.	For LOMR/CLOMR requests, do Base Flood	Elevations (BFEs) inc	crease?		⊠ Yes □ No			
	 a. For CLOMR requests, if either of th regulations: The proposed project encroaches 		•	·		the NFIP		
	The proposed project encroaches					0 foot.		
	 For LOMR requests, does this request If Yes, please attach proof of property notification can be found in the MT-2 Formula 	ty owner notification	·			perty owner		
2.	Does the request involve the placement or p	proposed placement of	f fill?		☐ Yes ⊠ No			
	If Yes, the community must be able to cerproposed structures, meets all of the standa NFIP regulations set forth at 44 CFR 60.3(a	dards of the local flood	dplain ordinances, and is	is reasonably safe from	flooding in accordan			
3.	For LOMR requests, is the regulatory floodw	ay being revised?			⊠ Yes □ No			
	If Yes, attach evidence of regulatory flood required for requests involving revisions to the [studied Zone A designation] unless a regular can be found in the MT-2 Form 2 Instructions	he regulatory floodway atory floodway is being	y. (Not required for revis	isions to approximate 1%	%-annual-chance floo	odplains		
4.	For LOMR/CLOMR requests, does this requests	est have the potential	to impact an endanger	ed species?	☐ Yes ⊠ No			
	If Yes, please submit documentation to the (ESA). Section 9 of the ESA prohibits an species, a permit is required from U.S. Fish	nyone from "taking" o	or harming an endange	ered species. If an acti	ion might harm an e			

For actions authorized, funded, or being carried out by Federal or State agencies, please submit documentation from the agency showing its compliance with Section 7(a)(2) of the ESA.

^{*} Not inclusive of all applicable regulatory requirements. For details, see 44 CFR parts 60 and 65.

U.S. DEPARTMENT OF HOMELAND SECURITY - FEDERAL EMERGENCY MANAGEMENT AGENCY

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	Flooding Source: Esperero Wash Note: Fill out one form for each flooding source studied						
			A. HYD	ROLOGY			
1.	Reason for New Hydrologic Ana	alysis (check	all that apply)				
	☐ Not revised (skip to section	B)	☐ No existing analysis	;		ta	
			☐ Proposed Condition	s (CLOMR)	☐ Changed phy	sical condition of watershed	
2.	Comparison of Representative	1%-Annual-Ch	ance Discharges				
	Location	Drain	age Area (Sq. Mi.)	Effective/F	FIS (cfs)	Revised (cfs)	
d/s	d/s of Thimble View Way 5.9 11037 10762						
d/s	d/s of Sunrise Dr. 6.11 9116 9170						
u/s	u/s of Ventana Cyn Wash 6.19 8440 8898						
3.	Methodology for New Hydrologic	c Analysis (cł	neck all that apply)				
	Statistical Analysis of Gage Regional Regression Equation		☑ Precipitation/Runoff☐ Other (please attack				
	Please enclose all relevant mod the new analysis.	lels in digital fo	ormat, maps, computatio	ns (including comput	ation of parameter	s) and documentation to support	
4.	Review/Approval of Analysis						
	If your community requires a reg	gional, state, o	or federal agency to revie	w the hydrologic ana	lysis, please attacl	n evidence of approval/review.	
5.	Impacts of Sediment Transport	on Hydrology					
	Was sediment transport consideration your explanation for why sediment			en fill out Section F (Sediment Transpo	rt) of Form 3. If No, then attach	

B. HYDRAULICS

1.	Reach to be Revised						
		Description	Cross Section	Water-Surface Elevations (ft.)			
				Effective	Proposed/Revised		
	Downstream Limit	convfluence Ventana Cyn Wash	RS 1	~2678.9 NGVD	2679.2 NGVD		
	Upstream Limit	of detailed study	RS 25	n/a	3070.9 NGVD		
2.	Hydraulic Method/Model Used						
	HEC-RAS version 4						

		B. HYDRAU	LICS (CONTINUED)			
3.	Pre-Submittal Review of Hydraulic Models					
	DHS-FEMA has developed two review progrespectively. These review programs may NFIP requirements, and that the data are cidentify areas of potential error or concerdownloaded from http://www.fema.gov/plan/CHECK-2 and CHECK-RAS. Review of you	help verify that the hycomparable with the a rn. These tools do /prevent/fhm/frm_soft.s	nydraulic estimates and assumptions and limitate not replace enginee shtm. We recommend	I assumptions in the montrions of HEC-2/HEC-RAPTING judgment. CHE I that you review your H	odel data are in acco AS. CHECK-2 and C ECK-2 and CHECK-F IEC-2 and HEC-RAS	ordance with CHECK-RAS RAS can be models with
4.	Models Submitted	<u>Natura</u>	al Run	Floodwa	ay Run	<u>Datum</u>
NAV	Duplicate Effective Model* Corrected Effective Model* Existing or Pre-Project Conditions Model Revised or Post-Project Conditions Model VD	File Name: File Name: File Name: File Name: Ventana	Plan Name: Plan Name: Plan Name: a Plan Name:	File Name:	Plan Name: Plan Name: Plan Name: a Plan Name:	<u> </u>
1	Other - (attach description)	File Name:	Plan Name:	File Name:	Plan Name:	
* Fo	or details, refer to the corresponding section of	of the instructions.				
<u> </u>		□ Digital Models	s Submitted? (Required	i)		
		C. MAPPING	G REQUIREMENTS	-	=	
prop flood indid requ	pertified topographic map must be submitted posed conditions 1%-annual-chance floodplay deplains and regulatory floodway (for detailed icated; stream, road, and other alignments uester's property; certification of a registered the referenced vertical datum (NGVD, NAVD)	lain (for approximate Zone AE, AO, and AF s (e.g., dams, levees I professional engineel	Zone A revisions) or H revisions); location and s, etc.); current commits.	the boundaries of the and alignment of all cross aunity easements and	1%- and 0.2%-ann s sections with station boundaries; boundaries	nual-chance ning control ries of the
į		□ Digital Mapping	(GIS/CADD) Data Subr	mitted		
mus to s	te that the boundaries of the existing or proposest tie-in with the effective floodplain and reguleshow the boundaries of the revised 1%- and ective 1%- and 0.2%-annual-chance floodplain	ilatory floodway bound nd 0.2%-annual-chanc n and regulatory floodw	daries. Please attach a ce floodplains and regu way at the upstream and	a copy of the effective ulatory floodway that tie d downstream limits of the	FIRM and/or FBFM, e-in with the bounda	, annotated
<u> </u>			M and/or FBFM (Requir	red)		
			LATORY REQUIREN	MENTS*		
1.	For LOMR/CLOMR requests, do Base Flood	Elevations (BFEs) inc	crease?		⊠ Yes □ No	
	 a. For CLOMR requests, if either of th regulations: The proposed project encroaches 		•	·		the NFIP
	The proposed project encroaches					0 foot.
	 For LOMR requests, does this request If Yes, please attach proof of property notification can be found in the MT-2 Formula 	ty owner notification	·			perty owner
2.	Does the request involve the placement or p	proposed placement of	f fill?		☐ Yes ⊠ No	
	If Yes, the community must be able to cerproposed structures, meets all of the standa NFIP regulations set forth at 44 CFR 60.3(a	dards of the local flood	dplain ordinances, and is	is reasonably safe from	flooding in accordan	
3.	For LOMR requests, is the regulatory floodw	ay being revised?			⊠ Yes □ No	
	If Yes, attach evidence of regulatory flood required for requests involving revisions to the [studied Zone A designation] unless a regular can be found in the MT-2 Form 2 Instructions	he regulatory floodway atory floodway is being	y. (Not required for revis	isions to approximate 1%	%-annual-chance floo	odplains
4.	For LOMR/CLOMR requests, does this requests	est have the potential	to impact an endanger	ed species?	☐ Yes ⊠ No	
	If Yes, please submit documentation to the (ESA). Section 9 of the ESA prohibits an species, a permit is required from U.S. Fish	nyone from "taking" o	or harming an endange	ered species. If an acti	ion might harm an e	

For actions authorized, funded, or being carried out by Federal or State agencies, please submit documentation from the agency showing its compliance with Section 7(a)(2) of the ESA.

^{*} Not inclusive of all applicable regulatory requirements. For details, see 44 CFR parts 60 and 65.

MT-2, Form 2 – Riverine Hydrology and Hydraulics (Supplemental Information)

Note: This supplemental information applies either in whole or in part to all flooding sources.

Section A, HYDROLOGY

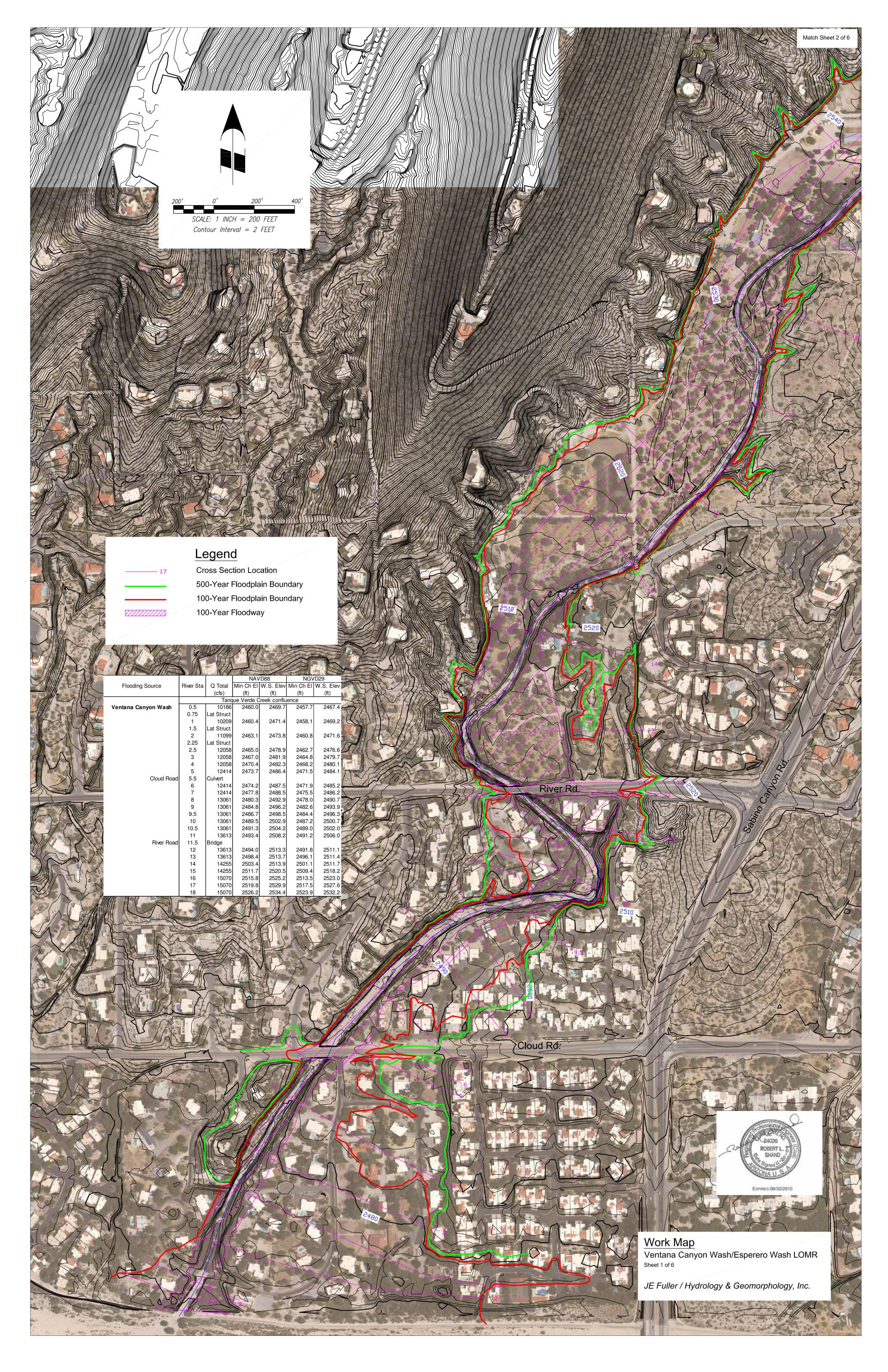
- Item 2, Comparison of Representative Discharges (see Table 1.1)
- Item 3, HEC-1 Model Submitted A total of thirty-two (32) individual HEC-1 models are provided one for each of the eight key concentration points or nodes and one for each of the four return intervals (10-, 50-, 100-, 500-year). The file name structure is as follows (example file name: 100N6-3.dat): 100 identifies the return interval; N6 identifies the concentration point or node; and, 3 denotes the 3-hour thunderstorm. The only exception is N61 which represents the combination of Node 6 and Node 11. The individual models, including the output, are provided in Appendix D. A watershed map showing the location of the concentration points is also provided in Appendix D.

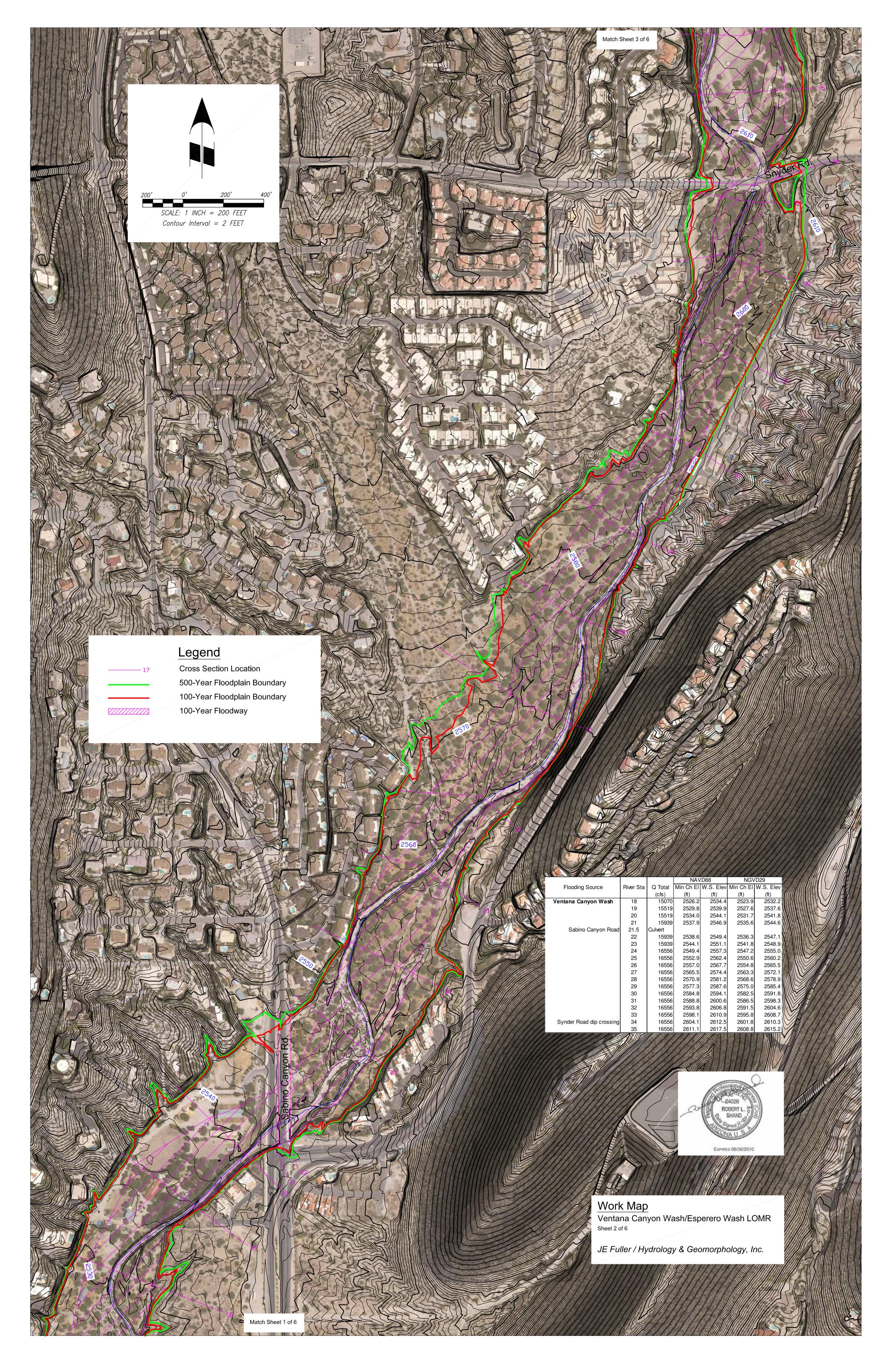
■ Section B, HYDRAULICS

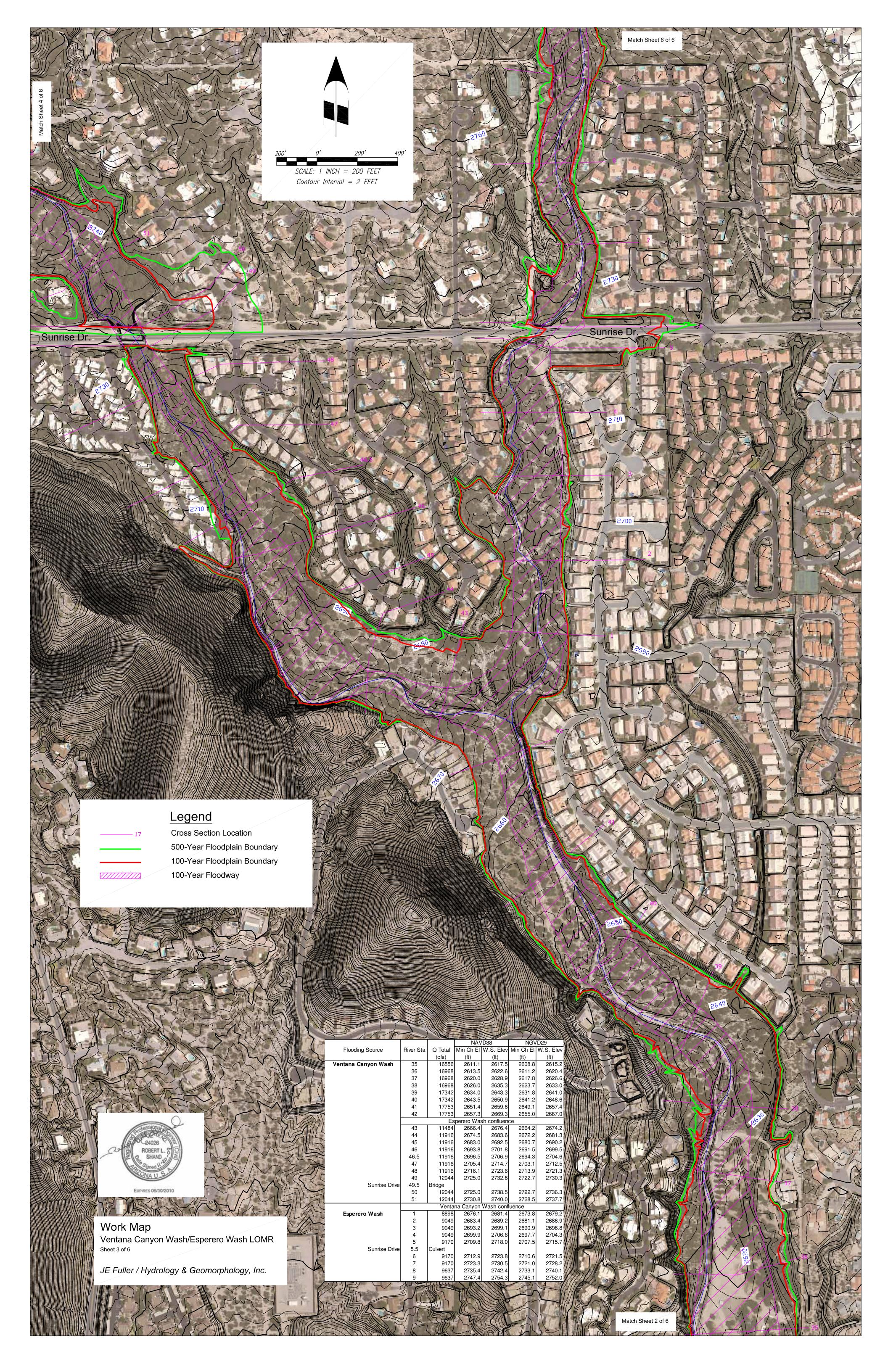
- Item 1, Reach to be Revised The entire reach of the Ventana Canyon Wash from its downstream limit at the Tanque Verde Wash confluence to its upstream limit at the Coronado National Forest boundary, and the entire reach of the Esperero Wash from its confluence with the Ventana Canyon Wash to a point located just south of the Coronado National Forest boundary.
- <u>Item 4, Models Submitted</u> The HEC-RAS project file name is Ventana.prj (see Appendix E). The project was divided into three reaches described as *Esperero Wash Reach-1*, which extends from the Ventana Canyon Wash confluence to the upstream limit of the study reach); *Ventana Canyon Lower Ventana*, which extends from the Tanque Verde Creek confluence to the Esperero Wash confluence; and, *Ventana Canyon Upper Ventana*, which extends from the Esperero Wash confluence to the upstream limit of the study reach. The project file includes two (2) plans. Plan 1 models the 10-, 50-, and 100-year discharges and the floodway. Plan 2 models the 500-year discharge.

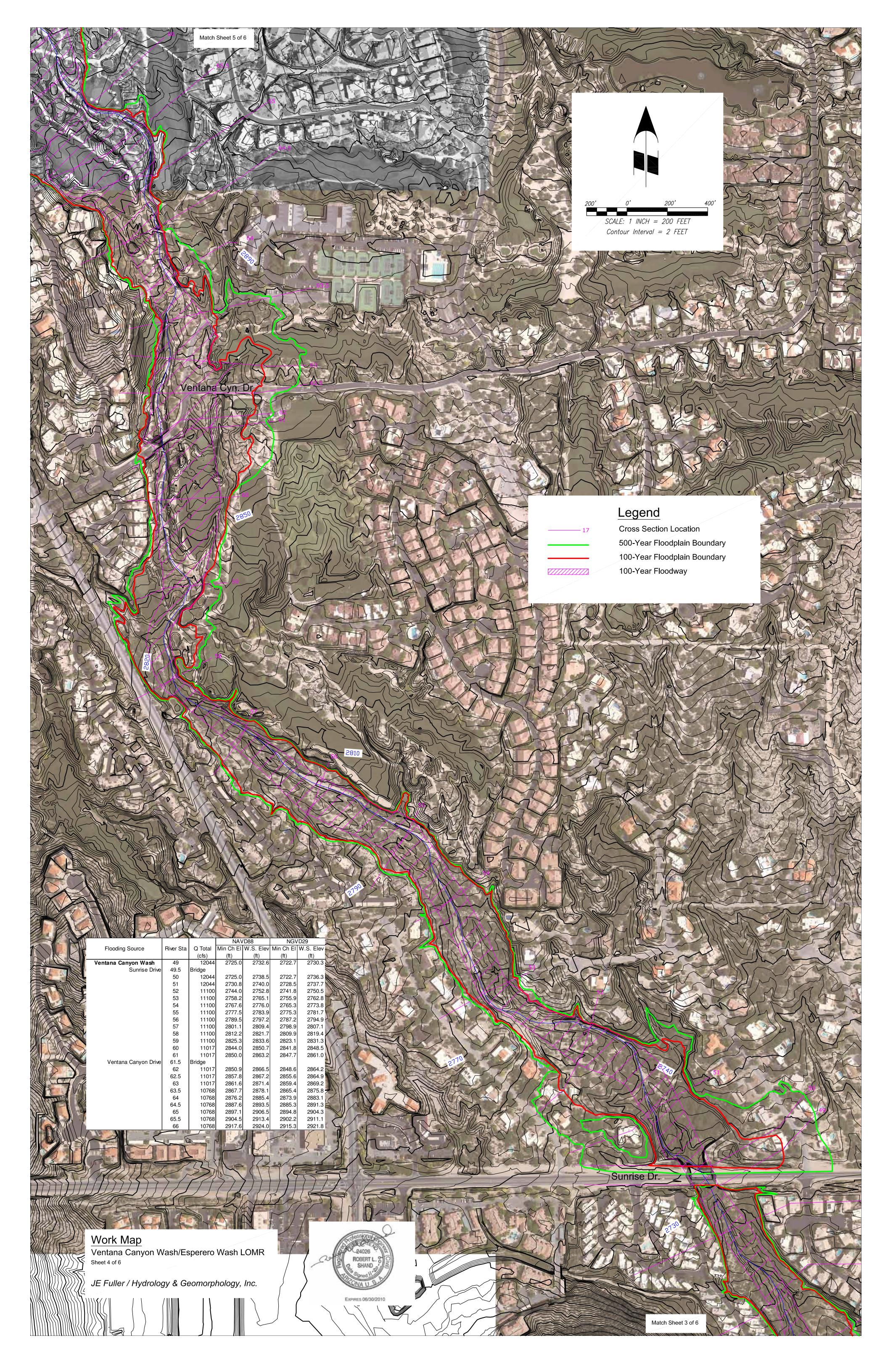
■ Section D, COMMON REGULATORY REQUIREMENTS

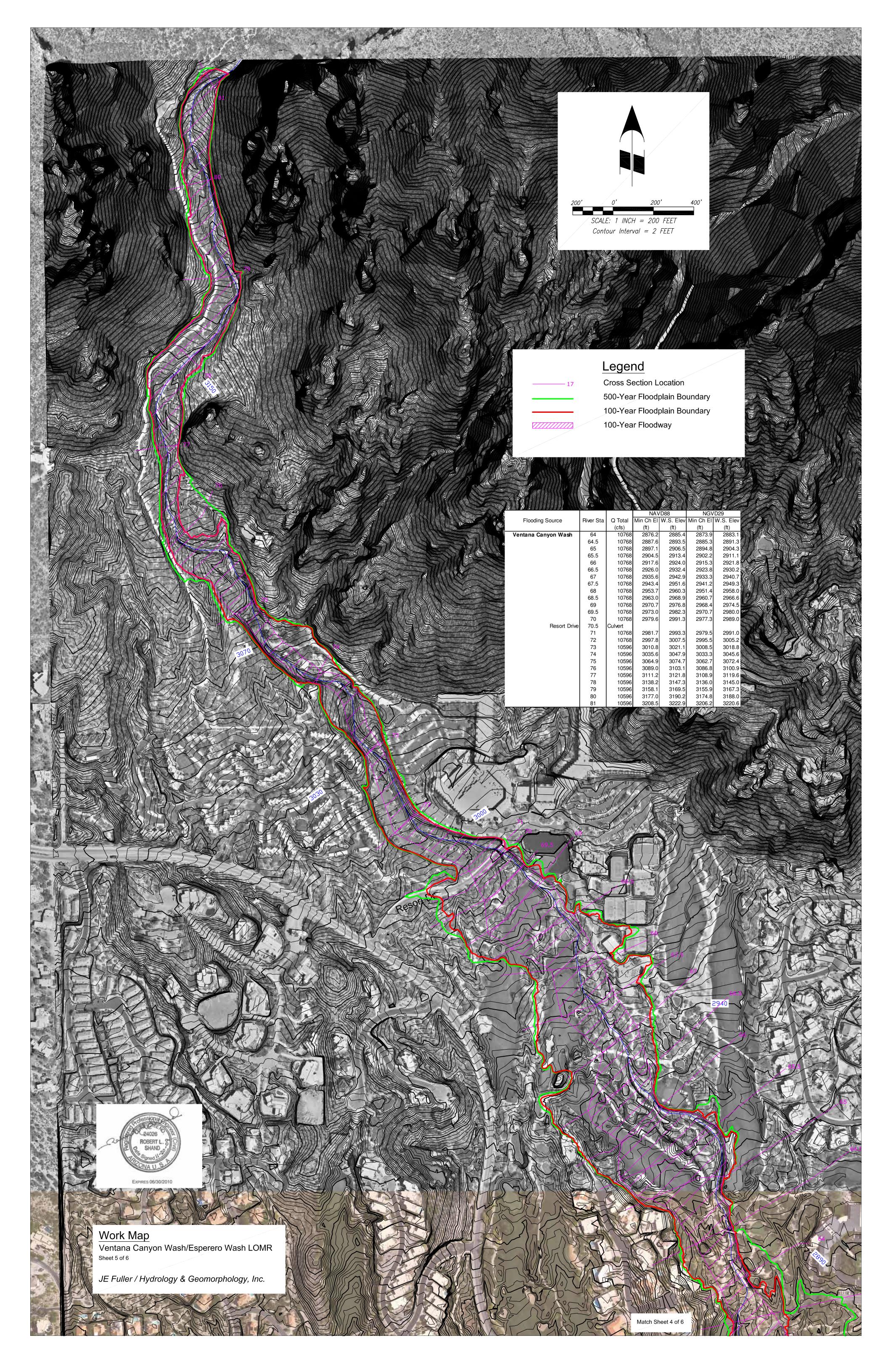
• <u>Item 1b and 3, Property Owner Notification</u> – The notification process will begin after any major concerns with the modeling have been adequately addressed.



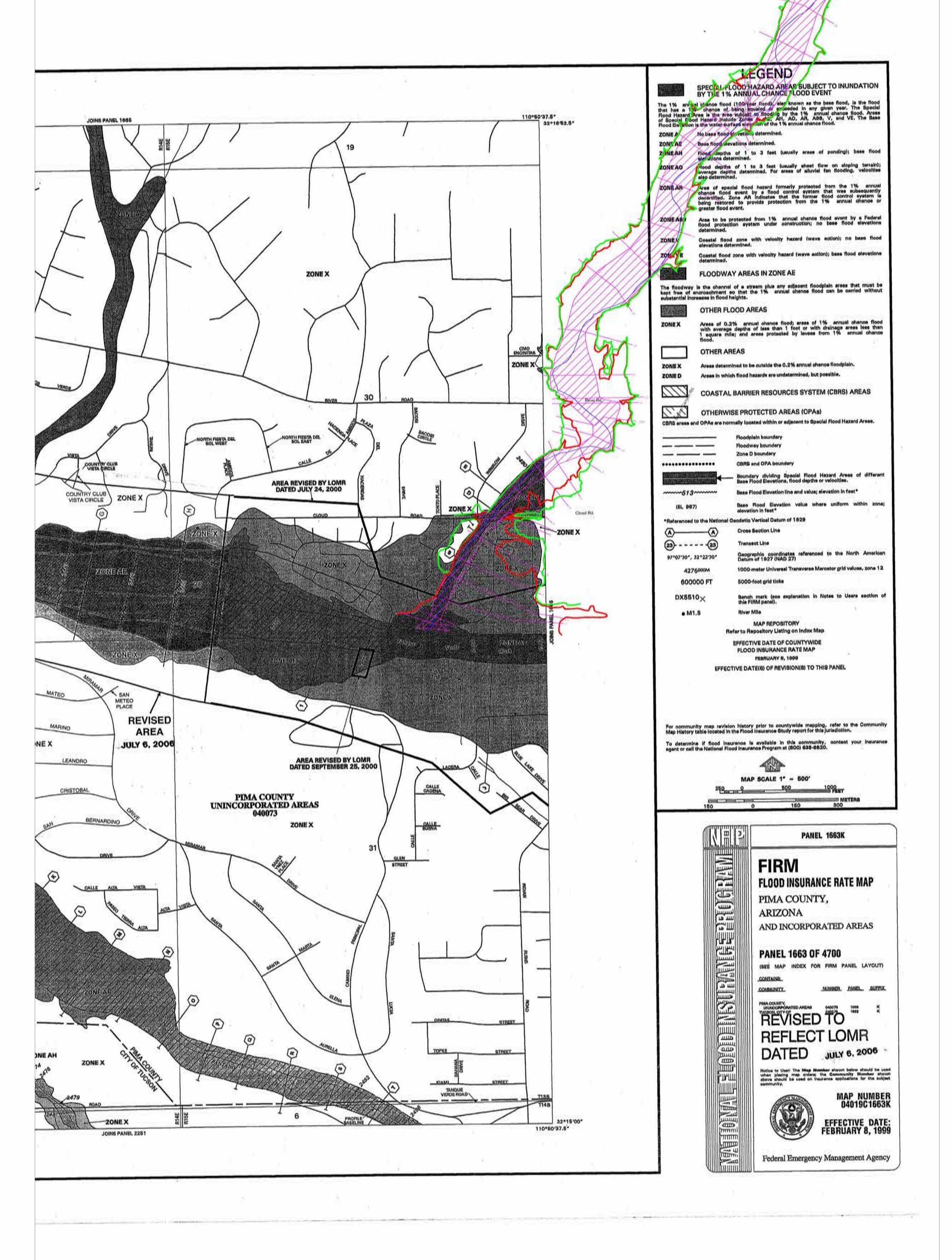












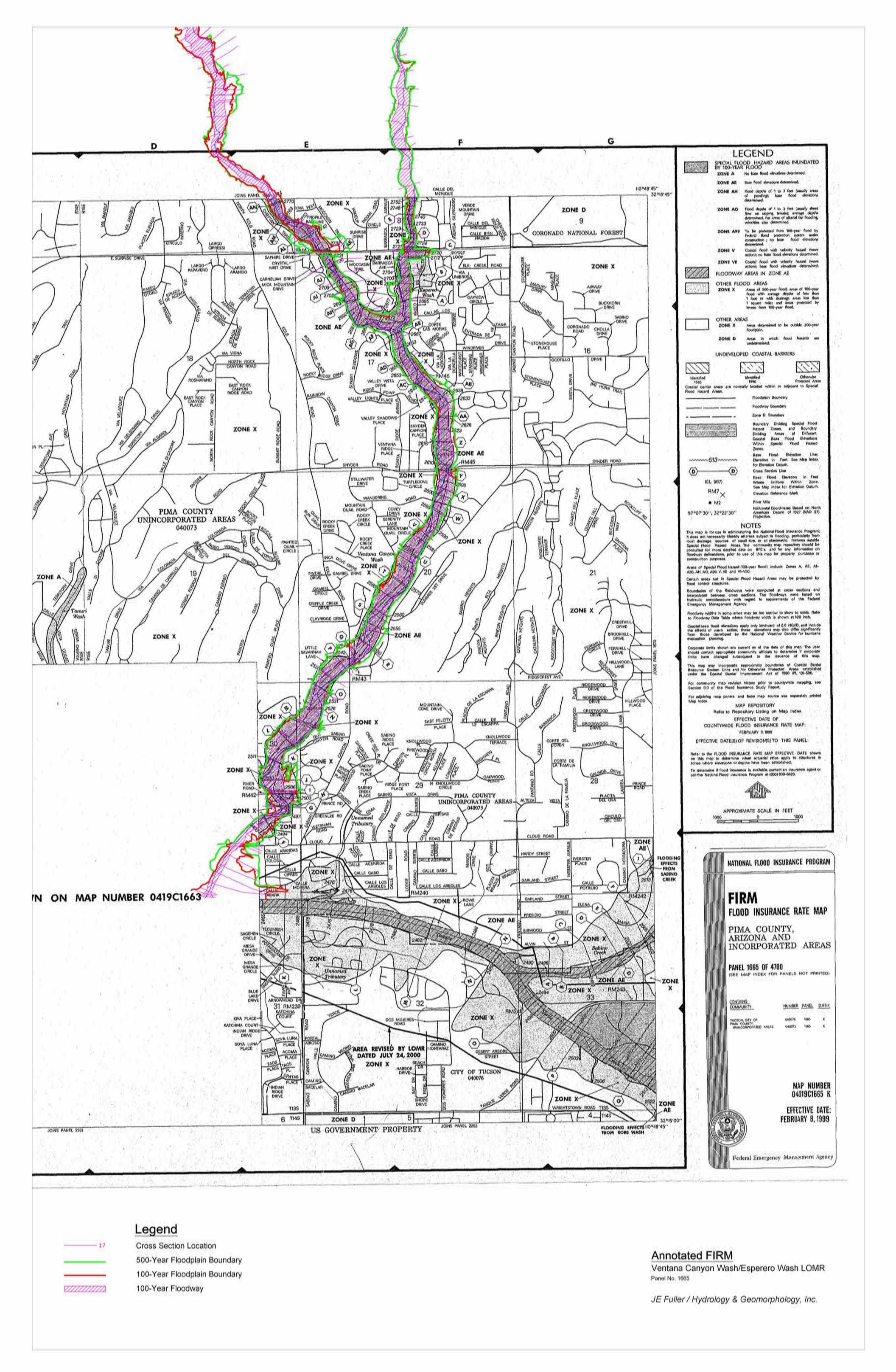
Legend

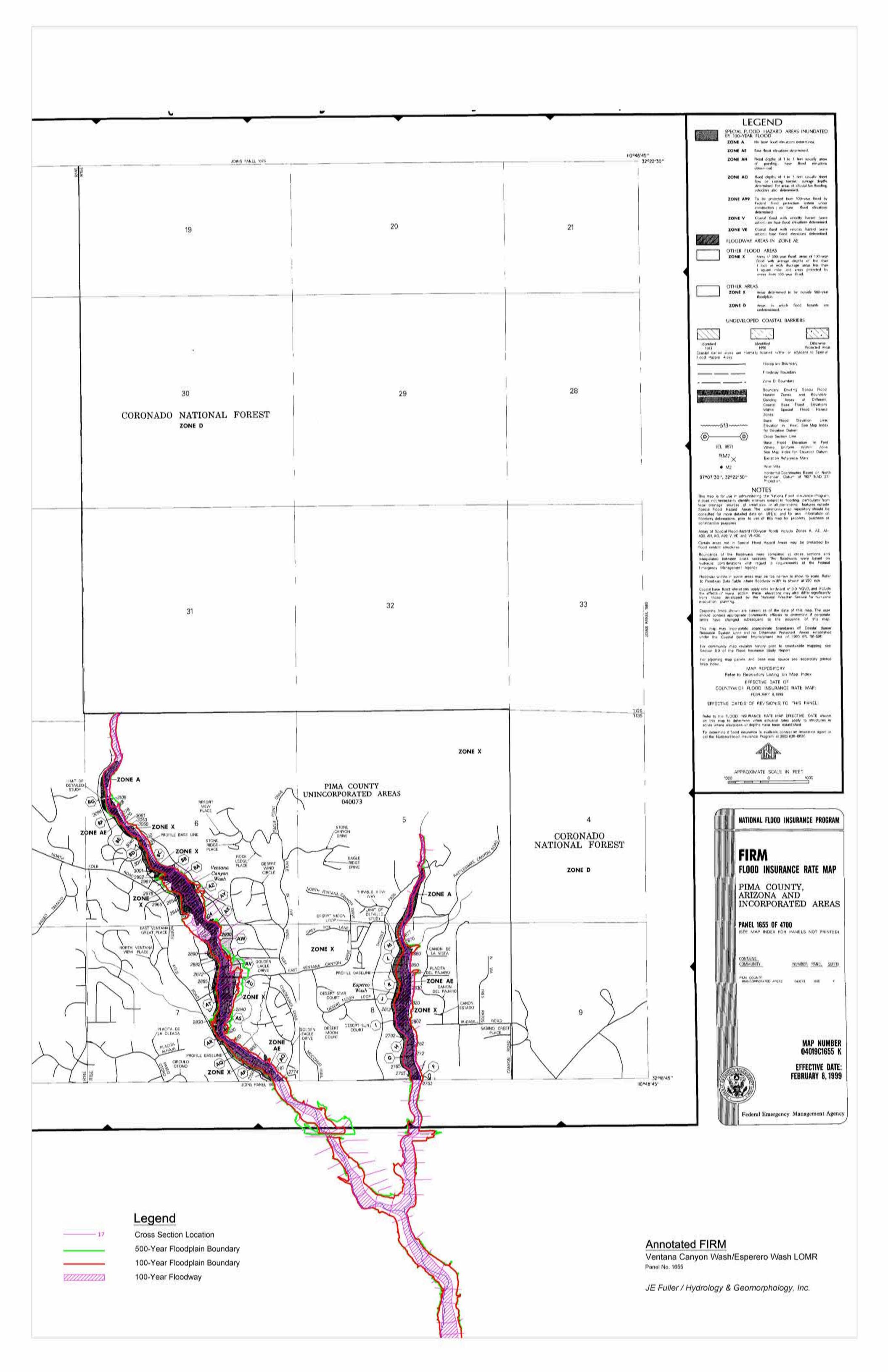
VIIIIIIII

Cross Section Location
500-Year Floodplain Boundary
100-Year Floodplain Boundary
100-Year Floodway

Annotated FIRM

Ventana Canyon Wash/Esperero Wash LOMR Panel No. 1663





U.S. DEPARTMENT OF HOMELAND SECURITY - FEDERAL EMERGENCY MANAGEMENT AGENCY

RIVERINE STRUCTURES FORM

O.M.B No. 1660-0016 Expires: 12/31/2010

PAPERWORK REDUCTION ACT

Public reporting burden for this form is estimated to average 7 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing, reviewing, and submitting the form. You are not required to respond to this collection of information unless a valid OMB control number appears in the upper right corner of this form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden to: Information Collections Management, U.S. Department of Homeland Security, Federal Emergency Management Agency, 500 C Street, SW, Washington DC 20472, Paperwork Reduction Project (1660-0016). Submission of the form is required to obtain or retain benefits under the National Flood Insurance Program. **Please do not send your completed survey to the above address.**

Flooding Source: Ventana Canyon Wash	
Note: Fill out one form for each flooding source studied	

A GENERAL

			A. GENERAL										
Comp	lete the appropriate sect	tion(s) for each Structure lister	d below:										
	Channelizationcomplete Section B Bridge/Culvertcomplete Section C Dam/Basincomplete Section D Levee/Floodwallcomplete Section E Sediment Transportcomplete Section F (if required)												
Descr	escription Of Structure												
1.	Name of Structure: C	loud Road culvert											
	Type (check one):	☐ Channelization	□ Bridge/Culvert	Levee/Floodwall	☐ Dam/Basin								
	Location of Structure:	at Cloud Road											
	Downstream Limit/Cros	ss Section: 5											
	Upstream Limit/Cross S	Section: 6											
2.	Name of Structure: S	abino Canyon Road bridge											
	Type (check one):	☐ Channelization	☐ Bridge/Culvert	Levee/Floodwall	☐ Dam/Basin								
	Location of Structure:	at Sabino Canyon Road											
	Downstream Limit/Cros	ss Section: 21											
	Upstream Limit/Cross S	Section: 22											
3.	Name of Structure:												
	Type (check one)	☐ Channelization	☐ Bridge/Culvert	☐ Levee/Floodwall	☐ Dam/Basin								
	Location of Structure:												
	Downstream Limit/Cros	ss Section:											
	Upstream Limit/Cross S	Section:											
NOT	E: For more structur	res, attach additional paç	ges as needed.		_								

B. CHANNELIZATION

F100	ding Source:									
Nam	ne of Structure:									
1.	Accessory Structures									
	The channelization includes (check one):									
	 Levees [Attach Section E (Levee/Floodwall)] Superelevated sections Debris basin/detention basin [Attach Section D (Dam/Basin)] Other (Describe): 	 □ Drop structures □ Transitions in cross sectional geometry □ Energy dissipator 								
2.	<u>Drawing Checklist</u>									
	Attach the plans of the channelization certified by a registered profession	onal engineer, as described in the instructions.								
3.	Hydraulic Considerations									
	The channel was designed to carry (cfs) and/or the -year	flood.								
	The design elevation in the channel is based on (check one):									
	☐ Subcritical flow ☐ Critical flow ☐	Supercritical flow Energy grade line								
	If there is the potential for a hydraulic jump at the following locations, cois controlled without affecting the stability of the channel.	neck all that apply and attach an explanation of how the hydraulic jump								
	☐ Inlet to channel ☐ Outlet of channel ☐ At Drop Structures ☐ Other locations (specify):	☐ At Transitions								
4.	Sediment Transport Considerations									
	Was sediment transport considered? ☐ Yes ☐ No If Yes, then fill out Section F (Sediment Transport). If No, then attach your explanation for why sediment transport was not considered.									
	C. BRIDGE/C	CULVERT								
Floo	C. BRIDGE/0	CULVERT								
		CULVERT								
	ding Source: Ventana Canyon Wash	CULVERT								
Nam	ding Source: Ventana Canyon Wash ne of Structure: Cloud Road culvert	CULVERT								
Nam	ding Source: Ventana Canyon Wash ne of Structure: Cloud Road culvert 1. This revision reflects (check one): Bridge/culvert not modeled in the FIS Modified bridge/culvert previously modeled in the FIS	ecial bridge routine, WSPRO, HY8): HEC-RAS								
Nam	ding Source: Ventana Canyon Wash ne of Structure: Cloud Road culvert 1. This revision reflects (check one): Bridge/culvert not modeled in the FIS Modified bridge/culvert previously modeled in the FIS Revised analysis of bridge/culvert previously modeled in the FIS 2. Hydraulic model used to analyze the structure (e.g., HEC-2 with sp. If different than hydraulic analysis for the flooding source, justify why the	ecial bridge routine, WSPRO, HY8): HEC-RAS hydraulic analysis used for the flooding source could not analyze the								
Nam	ding Source: Ventana Canyon Wash ne of Structure: Cloud Road culvert 1. This revision reflects (check one): Bridge/culvert not modeled in the FIS Modified bridge/culvert previously modeled in the FIS Revised analysis of bridge/culvert previously modeled in the FIS 2. Hydraulic model used to analyze the structure (e.g., HEC-2 with sp. If different than hydraulic analysis for the flooding source, justify why the structures. Attach justification. Attach plans of the structures certified by a registered professional engir (check the information that has been provided): Dimensions (height, width, span, radius, length) E. Shape (culverts only) L. Company Material T. Company Beveling or Rounding Si Wing Wall Angle Si	ecial bridge routine, WSPRO, HY8): HEC-RAS hydraulic analysis used for the flooding source could not analyze the								
Nam	ding Source: Ventana Canyon Wash ne of Structure: Cloud Road culvert 1. This revision reflects (check one): Bridge/culvert not modeled in the FIS Modified bridge/culvert previously modeled in the FIS Revised analysis of bridge/culvert previously modeled in the FIS 2. Hydraulic model used to analyze the structure (e.g., HEC-2 with sp. If different than hydraulic analysis for the flooding source, justify why the structures. Attach justification. Attach plans of the structures certified by a registered professional engir (check the information that has been provided): Dimensions (height, width, span, radius, length) E. Shape (culverts only) L. Material T. Beveling or Rounding Si Wing Wall Angle Si Skew Angle C.	necial bridge routine, WSPRO, HY8): HEC-RAS hydraulic analysis used for the flooding source could not analyze the neer. The plan detail and information should include the following rosion Protection by Chord Elevations – Upstream and Downstream op of Road Elevations – Upstream and Downstream ructure Invert Elevations – Upstream and Downstream ream Invert Elevations – Upstream and Downstream								

MT-2, Form 3 – Riverine Structures (Supplemental Information)

Note: This supplemental information applies to the Ventana Canyon Wash. No new structures exist along Esperero Wash flooding source.

■ Section A, GENERAL

 <u>Description of Structures</u> – Ventana Canyon Wash – Structure No. 4: Name: Resort Drive culvert; Type: Bridge/Culvert; Location of Structure: at Resort Drive; Downstream Cross Section 70; Upstream Cross Section 71.

■ Section C, BRIDGE/CULVERT

 <u>Item 3, Plans of Structures</u> – The information checked in this section was obtained from using a combination of the field survey notes, which includes sketches and an AutoCAD drawing file, and the actual plan sheets. Copies of this information are provided in Appendix C.

III. MAPPING AND SURVEY INFORMATION

3.1 Field Survey

Stantec Consulting conducted a field survey to define the geometry of all existing bridge openings and/or culvert crossings along the project reach. The survey included the following: (1) cross sections within five feet of the upstream and downstream face of each structure; (2) bed elevations at the base of each pier along the upstream and downstream face of each bridge; (3) vertical profiles of the bridge at the each pier, including a definition of the shape and the associated low cord elevation; (4) cross reference points in common with the Pima Association of Governments (PAG) topography; and (5) monuments at each structure. The basis of elevation was the North American Vertical Datum of 1988 (NAVD 88). Copies of the survey notes and an AutoCAD drawing file from Stantec Consulting are provided in Appendix C. In addition, a field reconnaissance was conducted to verify channel characteristics and to collect data for determining Manning's n-value. Photographs taken at various locations along the study reach are provided in Appendix C.

3.2 Mapping and Records

The hydraulic analysis was conducted using a combination of 1998 and 2000, 1' = 100', two-foot contour interval mapping provided by Pima County for this study. The horizontal control was based on the North American Datum of 1983 (NAD 1983). As previously noted, the vertical control was based on the North American Vertical Datum of 1988 (NAVD 1988).

IV. HYDROLOGY

4.1 Method Description

The regulatory discharges associated with this request were computed using HEC-1 (Reference 4). The base model, which was initially prepared by Tetra Tech, Inc. in 2002 (Reference 5), was provided by the RFCD. A copy of the Tetra Tech's summary report is provided in Appendix D. However, Tetra Tech's base model was limited to the 100-year return interval. As part of this LOMR request, per the Pima County's current hydrologic modeling criteria, JEF updated the base model to incorporate (1) the new NOAA Atlas Volume 14 data; (2) the 3-hour, upper 90% confidence interval precipitation values from NOAA 14; (3) the Hydro-40 aerial reduction factors, and (4) the SCS Type II distribution. In addition, JEF expanded the modeling effort to included the 10-, 50-, 100-, and 500-year return intervals. A total of ten point-precipitation-value data sets were obtained from the NOAA 14 web site based on the longitude and latitude of the centroid of each the ten major sub-basins. However, since the range of individual values was narrow, an average value was selected for each return interval. An areal reduction factor was then applied to define a basin average value for each of the ten major sub-basins. The results of the HEC-1 analysis are summarized in Table 1.1. In addition, Table 1.1 provides a comparison between the effective discharges and the revised discharges. The revised HEC-1 models are provided in Appendix D.

Subsection	Requested Information	Description/Location
4.1	Method Description	Thirty-two HEC-1 models – separate models for each return interval at eight nodes (4, 6, 8, 10, 11, 6-11, 14, and 15).
4.2	Parameter Estimation	
4.2.1	Drainage Area Boundaries	Watershed Map, Reference 5
4.2.2	Watershed Work Maps	Reference 5
4.2.3	Gage Data	n/a
4.2.4	Statistical Parameters	n/a
4.2.5	Precipitation	Reference 6
4.2.6	Physical Parameters	Reference 5
4.3	Special Problems, Solution, Modeling Messages	none
4.4	Calibration	n/a
4.5	Final Results	
4.5.1	Hydrologic Analysis Results	Table 1.1 in Section I
4.5.2	Verification of results	n/a

V. HYDRAULICS

The majority of the information that pertains to this section is provided on FEMA MT-2 Form 2 or on the supplemental information sheet that accompanies that form. Using the outline provided in State Standard 1-97 – *Instructions for Organizing and Submitting Technical Documentation for Flood Studies*, the following table briefly describes the information requested and/or indicates its location in the TDN.

Subsection	Requested Information	Description/Location
5.1	Method Description	FEMA Forms
5.2	Work Study Maps	Attached to MT-2 Form 2
5.3	Parameter Estimation	
5.3.1	Roughness Coefficients	Reference 8
5.3.2	Expansion and Contraction Coefficients	Typical values for uniform reaches and bridges were applied.
5.4	Cross Section Description	New placement and orientation along majority of revised reach. All new sections from DTM and topographic mapping.
5.5	Modeling Considerations	
5.5.1	Hydraulic Jump and Drop Analysis	n/a
5.5.2	Bridges and Culverts	Special Bridge and Culvert Methods in HEC-RAS models.
5.5.3	Levees and Dikes	n/a
5.5.4	Islands and Flow Splits	n/a
5.5.5	Ineffective Flow Areas	Overbank depressions and/or low areas and areas immediately upstream and downstream of bridge crossings
5.6	Floodway Modeling	Method 4 followed by Method 1
5.7	Problems Encountered During the Study	none
5.7.1	Special Problems and Solutions	none
5.7.2	Modeling Warning and Error Messages	Only typical messages that are not significant to results obtained.
5.8	Calibration	n/a
5.9	Final Results	
5.9.1	Hydraulic Analysis Results	Appendix E
5.9.2	Verification of Results	n/a

VI. SEDIMENT TRANSPORT/EROSION

No sediment transport study was conducted for this project.

VII. FIS REPORT DATA

Subsection	Requested Information	Description/Location
7.1	Summary of Discharges	See Table 7.1
7.2	Floodway Data	See Table 7.2
7.3	Annotated FIRM	Attachment MT-2 Form 2
7.4	Flood Profiles	Appendix B

Table 7.1 Revised FIS Summary of Discharges

		Drainage	ainage Peak Discharges (cfs)				
Flooding Source	Location	Area	10-year	50-year	100-year	500-year	
		(sq mi)	(in)	(in)	(cfs)	(cfs)	
	downstream of Thimble View Way	5.9	5121	8907	10762	15953	
Esperero Wash	upstream of Sunrise Drive	6.11	4333	7067	9170	13663	
1	upstream of confluence with Ventana						
	Canyon Wash	6.19	4246	6949	8898	13574	
	upstream of Resort Drive	3.85	5179	8813	10596	14864	
	upstream of Sunrise Drive	6.98	5378	9448	12044	17805	
	upstream of confluence with Esperero Wash	7.94	5271	9151	11484	17544	
Ventana Canyon	downstream of confluence with						
Wash	Esperero Wash	14.14	8122	14053	17753	27253	
	upstream of Sabino Canyon Road	15.87	7271	12547	15939	25162	
	downstream of River Road		5325	9453	12058	19072	
	at confluence with Tanque Verde						
	Creek	16.64	5066	9030	11527	18238	

Table 7.2 Floodway Data

Flooding Source				Floodway		Base Flood Water Surface Elevation				
Watercourse	Cross	Distance ¹	Distance ¹	Width	Section	Mean	Regulatory	Without	With	Increase
	Section				Area	Velocity	W.S. Elev	Floodway	Floodway	
		(ft)	(mi)	(ft)	(sq ft)	(ft/s)	(ft)	(ft)	(ft)	(ft)
Esperero Wash	1	640	0.121	276	938.3	9.5	2681.4	2681.4	2681.9	0.5
	2	990	0.188	228	849.6	10.7	2689.2	2689.2	2689.3	0.2
	3	1400	0.265	267	892.2	10.1	2699.1	2699.1	2699.1	0.0
	4	1720	0.326	284	932.8	9.7	2706.6	2706.6	2706.6	0.0
	5	2060	0.390	169	762.0	12.0	2718.0	2718.0	2718.0	0.0
	6	2185	0.414	681	1894.7	8.1	2723.8	2723.8	2723.8	0.0
	7	2605	0.493	152	735.6	12.5	2730.5	2730.5	2730.5	0.0
	8	3025	0.573	169	789.7	12.2	2742.4	2742.4	2742.4	0.0
	9	3445	0.652	177	807.2	11.9	2754.3	2754.3	2754.3	0.0
	10	3810	0.722	223	876.4	11.0	2765.7	2765.7	2765.7	0.0
	11	4250	0.805	269	1060.7	9.7	2780.3	2780.3	2780.3	0.0
	12	4755	0.901	154	817.5	12.6	2795.1	2795.1	2795.1	0.0
	13	5055	0.957	302	1049.0	9.8	2806.6	2806.6	2806.6	0.0
	14	5455	1.033	328	1138.1	9.0	2819.0	2819.0	2819.4	0.4
	15	5905	1.118	223	947.5	11.4	2835.9	2835.9	2836.2	0.3
	16	6315	1.196	248	1057.7	10.2	2848.8	2848.8	2849.2	0.4
	17	6715	1.272	121	800.5	13.4	2861.9	2861.9	2862.1	0.2
	17.1	6750	1.278	89	519.3	13.7	2863.2	2863.2	2863.5	0.3
	18	7125	1.349	65	476.4	14.9	2877.6	2877.6	2877.7	0.2
	19	7525	1.425	54	438.9	16.2	2899.1	2899.1	2899.6	0.5
	20	7940	1.504	98	558.4	12.7	2928.2	2928.2	2928.2	0.0
	21	8350	1.581	71	491.3	14.4	2960.8	2960.8	2960.8	0.0
	22	8765	1.660	58	457.5	15.5	2994.2	2994.2	2994.2	0.0
	23	9205	1.743	87	525.8	13.5	3027.0	3027.0	3027.0	0.0
	24	9545	1.808	82	502.3	14.1	3050.0	3050.0	3050.0	0.0
	25	9855	1.866	68	472.8	15.0	3073.1	3073.1	3073.1	0.0

¹ miles above confluence with Ventana Canyon Wash

Table 7.2 Floodway Data

Floo	oding Source	ce			Floodway		Base Flood Water Surface Elevation			
Watercourse	Cross	Distance ¹	Distance ¹	Width	Section	Mean	Regulatory	Without	With	Increase
	Section				Area	Velocity	W.S. Elev	Floodway	Floodway	
		(ft)	(mi)	(ft)	(sq ft)	(ft/s)	(ft)	(ft)	(ft)	(ft)
Ventana Canyon Wash	0.5	0	0	168	929.4	10.8	2469.7	2469.7	2469.9	0.2
	1	85	0.016	119	806.3	12.5	2471.4	2471.4	2471.8	0.4
	2	301.69	0.057	128	759.1	14.6	2473.8	2473.8	2473.8	0.0
	2.5	581.69	0.110	150	890.3	13.5	2478.9	2478.9	2478.9	0.0
	3	806.69	0.153	158	1337.0	9.0	2481.9	2481.9	2481.9	0.0
	4	1123.28	0.213	115	774.4	15.6	2482.3	2482.3	2482.3	0.0
	5	1363.28	0.258	249	1520.4	8.2	2486.4	2486.4	2486.4	0.0
	6	1438.28	0.272	254	1715.7	7.2	2487.5	2487.5	2488.4	0.9
	7	1803.28	0.342	108	794.0	15.6	2488.5	2488.5	2488.5	0.0
	8	2157.67	0.409	195	1116.4	11.7	2492.9	2492.9	2492.9	0.0
	9	2391.08	0.453	161	1045.7	12.5	2496.2	2496.2	2496.4	0.2
	9.5	2681.08	0.508	111	961.8	13.6	2498.5	2498.5	2499.5	0.9
	10	3041.08	0.576	224	1828.8	7.1	2502.9	2502.9	2503.0	0.1
	10.5	3351.08	0.635	304	1443.6	9.9	2504.2	2504.2	2504.7	0.5
	11	3971.24	0.752	558	1976.6	6.9	2508.2	2508.2	2508.9	0.7
	12	4049.37	0.767	383	3328.7	4.1	2513.3	2513.3	2513.5	0.2
	13	4429.77	0.839	506	4527.1	3.0	2513.7	2513.7	2514.1	0.4
	14	5054.96	0.957	223	1303.1	10.9	2513.9	2513.9	2514.3	0.4
	15	5685.82	1.077	490	2663.0	5.4	2520.5	2520.5	2521.4	0.9
	16	6157.08	1.166	397	1798.5	8.4	2525.2	2525.2	2525.3	0.1
	17	6694.84	1.268	357	2082.6	7.2	2529.9	2529.9	2530.5	0.6
	18	7278.79	1.379	340	1746.1	8.6	2534.4	2534.4	2535.1	0.7
	19	7792.94	1.476	368	1983.5	7.8	2539.9	2539.9	2540.6	0.7
	20	8156.36	1.545	292	1603.7	9.7	2544.1	2544.1	2544.9	0.8
	21	8379.5	1.587	273	1902.3	10.2	2546.9	2546.9	2547.7	0.8
	22	8443.51	1.599	298	2452.3	7.0	2549.4	2549.4	2550.2	0.8
	23	8977	1.700	320	1601.0	10.0	2551.1	2551.1	2551.8	0.6
	24	9541.37	1.807	361	1796.0	9.2	2557.3	2557.3	2557.8	0.5
	25	10058.71	1.905	292	1641.6	10.1	2562.4	2562.4	2563.4	1.0
	26	10530.25	1.994	319	1705.3	9.7	2567.7	2567.7	2568.3	0.5
	27	11110.98	2.104	341	1708.2	9.7	2574.4	2574.4	2574.5	0.1
	28	11632.31	2.203	291	1647.6	10.1	2581.2	2581.2	2582.0	8.0
	29	12133.13	2.298	315	1733.9	9.6	2587.6	2587.6	2587.8	0.2

¹ miles above confluence with Tanque Verde Creek

Table 7.2 Floodway Data

Floo	oding Source	ce			Floodway		Base Flood Water Surface Elevation			
Watercourse	Cross	Distance ¹	Distance ¹	Width	Section	Mean	Regulatory	Without	With	Increase
	Section				Area	Velocity	W.S. Elev	Floodway	Floodway	
		(ft)	(mi)	(ft)	(sq ft)	(ft/s)	(ft)	(ft)	(ft)	(ft)
Ventana Canyon Wash	30	12682.51	2.402	228	1422.6	11.6	2594.1	2594.1	2594.6	0.6
(continued)	31	13210.72	2.502	224	1457.0	11.4	2600.6	2600.6	2601.0	0.5
	32	13647.69	2.585	245	1545.8	10.7	2606.8	2606.8	2606.9	0.1
	33	13948.97	2.642	204	1432.3	11.6	2610.9	2610.9	2611.0	0.0
	34	14035.41	2.658	229	1746.2	9.5	2612.5	2612.5	2613.2	0.7
	35	14620.63	2.769	417	1680.6	9.9	2617.5	2617.5	2618.2	0.7
	36	14927.75	2.827	359	1753.9	9.7	2622.6	2622.6	2623.2	0.6
	37	15451.28	2.926	244	1426.9	11.9	2628.9	2628.9	2629.6	0.7
	38	15789.69	2.990	201	1355.3	12.5	2635.3	2635.3	2636.2	0.9
	39	16283.9	3.084	201	1322.5	13.1	2643.3	2643.3	2643.6	0.3
	40	16853.9	3.192	319	1731.2	10.0	2650.9	2650.9	2651.8	1.0
	41	17430.17	3.301	207	1397.2	12.7	2659.6	2659.6	2660.6	1.0
	42	18020.09	3.413	152	1225.8	14.5	2669.3	2669.3	2669.7	0.4
	43	18524.75	3.508	145	977.6	11.8	2676.4	2676.4	2677.0	0.6
	44	18966.38	3.592	101	790.3	15.1	2683.6	2683.6	2684.1	0.5
	45	19600.5	3.712	262	1239.2	9.6	2692.5	2692.5	2693.3	0.8
	46	20093.75	3.806	254	1196.9	10.0	2701.8	2701.8	2702.5	0.8
	46.5	20352.75	3.855	126	824.5	14.5	2706.9	2706.9	2707.4	0.5
	47	20638.75	3.909	120	852.1	14.0	2714.7	2714.7	2715.3	0.6
	48	21048.38	3.986	142	882.7	13.5	2723.6	2723.6	2724.6	1.0
	49	21298.79	4.034	118	809.4	14.9	2732.6	2732.6	2732.6	0.0
	50	21418.57	4.057	151	1542.3	7.8	2738.5	2738.5	2738.5	0.0
	51	21794.69	4.128	174	977.3	12.3	2740.0	2740.0	2740.0	0.0
	52	22361.86	4.235	208	1047.6	10.6	2752.8	2752.8	2753.4	0.7
	53	22942.31	4.345	147	841.5	13.2	2765.1	2765.1	2765.2	0.1
	54	23433.88	4.438	160	954.2	11.6	2776.0	2776.0	2776.9	0.9
	55	23853.88	4.518	189	899.4	12.3	2783.9	2783.9	2784.2	0.3
	56	24348.9	4.612	192	994.6	11.2	2797.2	2797.2	2797.9	0.7
	57	24822	4.701	118	782.5	14.2	2809.4	2809.4	2810.1	0.7
	58	25268.97	4.786	92	725.9	15.3	2821.7	2821.7	2822.4	0.7

¹ miles above confluence with Tanque Verde Creek

Table 7.2 Floodway Data

Flooding Source				Floodway			Base Flood Water Surface Elevation			
Watercourse	Cross	Distance ¹	Distance ¹	Width	Section	Mean	Regulatory	Without	With	Increase
	Section				Area	Velocity	W.S. Elev	Floodway	Floodway	
		(ft)	(mi)	(ft)	(sq ft)	(ft/s)	(ft)	(ft)	(ft)	(ft)
Ventana Canyon Wash	59	25775.08	4.882	195	959.8	11.6	2833.6	2833.6	2834.3	0.8
(continued)	60	26244.94	4.971	321	1189.2	9.3	2850.7	2850.7	2851.2	0.5
	61	26512.96	5.021	299	1471.9	9.2	2863.2	2863.2	2863.9	0.7
	62	26576.02	5.033	372	2510.7	5.3	2866.5	2866.5	2867.4	1.0
	62.5	26781.02	5.072	310	1225.8	9.0	2867.2	2867.2	2867.3	0.1
	63	26951.02	5.104	350	1367.4	8.1	2871.4	2871.4	2872.4	1.0
	63.5	27164.02	5.145	200	952.5	11.3	2878.1	2878.1	2878.3	0.3
	64	27375.02	5.185	144	860.1	12.5	2885.4	2885.4	2885.6	0.2
	64.5	27650.02	5.237	200	909.4	11.8	2893.5	2893.5	2894.1	0.5
	65	27911.02	5.286	313	1218.0	8.8	2906.5	2906.5	2907.1	0.6
	65.5	28163.02	5.334	414	1373.7	7.8	2913.4	2913.4	2914.4	1.0
	66	28438.02	5.386	234	971.2	11.1	2924.0	2924.0	2924.5	0.5
	66.5	28622.02	5.421	332	1149.0	9.4	2932.4	2932.4	2933.0	0.6
	67	28838.02	5.462	211	984.3	10.9	2942.9	2942.9	2943.3	0.3
	67.5	29062.02	5.504	288	1090.1	9.9	2951.6	2951.6	2951.9	0.4
	68	29267.02	5.543	253	1046.1	10.3	2960.3	2960.3	2960.7	0.5
	68.5	29530.02	5.593	339	1076.2	10.0	2968.9	2968.9	2969.4	0.6
	69	29768.02	5.638	418	1227.3	8.8	2976.8	2976.8	2977.8	1.0
	69.5	29883.02	5.660	271	1047.3	10.3	2982.3	2982.3	2982.6	0.3
	70	29988.02	5.680	228	1157.9	9.3	2991.3	2991.3	2991.9	0.7
	71	30030.02	5.688	213	1339.4	8.0	2993.3	2993.3	2994.2	0.9
	72	30500.02	5.777	133	837.4	12.9	3007.5	3007.5	3008.4	0.9
	73	30899.02	5.852	80	674.1	15.7	3021.1	3021.1	3021.8	0.8
	74	31410.02	5.949	72	669.7	15.8	3047.9	3047.9	3048.8	0.9
	75	31935.02	6.048	111	753.0	14.1	3074.7	3074.7	3075.1	0.4
	76	32484.02	6.152	233	1117.5	9.5	3103.1	3103.1	3103.1	0.0
	77	32957.02	6.242	109	754.8	14.0	3121.8	3121.8	3121.8	0.0
	78	33462.02	6.338	182	918.8	11.5	3147.3	3147.3	3147.3	0.0
	79	33967.02	6.433	135	852.0	12.4	3169.5	3169.5	3169.5	0.0
	80	34463.02	6.527	131	865.5	12.2	3190.2	3190.2	3190.2	0.0
	81	35011.02	6.631	110	800.2	13.2	3222.9	3222.9	3222.9	0.0

¹ miles above confluence with Tanque Verde Creek

APPENDIX A

References

TDN – VENTANA CANYON WASH/ESPERERO WASH LOMR

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APPENDIX B

General Documentation & Correspondence

APPENDIX C

Site Photographs & Survey Field Notes

APPENDIX D

Hydrologic Analysis – Summary of Supporting Documentation

APPENDIX E

Hydraulic Analysis – Summary of Supporting Documentation

Appendix F

GIS Shape Files