

# Del Cerro Wash Technical Data Notebook

Prepared for:

Pima County Regional Flood Control District  
97 East Congress, Tucson, AZ 85701

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**Appendix B: General Documentation and Correspondence**

**Appendix C: Survey Field Notes**

**Appendix D: Hydrologic Analysis, Supporting Documents**

**Appendix E: Hydraulic Analysis, Supporting Documents**

**Appendix F: Erosion Analysis, Supporting Documents**

**Exhibit**

Exhibit 1 100-yr floodplain limits for the Del Cerro Wash

Exhibit 2 Annotated Flood Insurance Rate Map for the Del Cerro Wash

**Attached CD** Del Cerro Wash TDN with supporting models and GIS data.

# Section 1 Introduction

## 1.1 Purpose

This Technical Data notebook (TDN) has been prepared for a Letter of Map Revision (LOMR) application for a portion of the Del Cerro Wash (DCR) located in Pima County, Arizona. The objective of the TDN and LOMR submission is to provide regulatory discharge rates and floodplain limits along the DCR using better topographic, hydrologic, and hydraulic data.

This TDN was prepared in accordance with the “Instructions for Organizing and Submitting Technical Documentation for Flood Studies” prepared by the Arizona Department of Water Resources, Flood Mitigation Section (Arizona State Standard, SSA 1-97) and FEMA Guideline. FEMA LOMR forms are included in this TDN.

## 1.2 Project Authority

The State of Arizona has delegated the responsibility to each county flood control district to adopt floodplain regulations designed to promote the public health, safety and general welfare of its citizenry as provided under the Arizona Revised Statutes, Title 48, Chapter 21, Article 1, Sections 48-3601 through 3627. More specifically, A.R.S. 3609 directs county flood control districts to adopt floodplain regulations that:

- A. Regulate all development of land, construction of residential, commercial or industrial structures or uses of any kind which may divert, retard or obstruct flood water and threaten public health or safety or the general welfare; and
- B. Establish minimum flood protection elevations and flood damage prevention requirements for uses, structures and facilities which are vulnerable to flood damage; and
- C. Comply with state and local land use plans and ordinances, if any.  
In conformance with A.R.S. 3609, this ordinance provides for protection of the public health safety and welfare by regulation of flood and erosion hazard areas to control flood hazards and prevent repetitive loss from flood damage.
- D. The flood hazard areas of Pima County are subject to periodic inundation which may result in loss of life and property, create health and safety hazards, disrupt commerce and governmental services, require extraordinary public expenditures for flood protection and relief, and impair the tax base, all of which adversely affect the public health, safety, and general welfare.
- E. These flood losses are caused by the cumulative effect of obstructions in areas of special flood hazards which increase flood heights, flow velocities, and cause flood and erosion damage. Uses that are inadequately flood-proofed, elevated, or otherwise protected from flood damage, also contribute to the flood loss. (Ord. 2005 FC-2 § 2 (part), 2005).

Section 16 of the Pima County Ordinance describes the provisions for floodplain regulation in Pima County.

This study has been prepared by the Pima County Regional Flood Control District (RFCD):

Pima County Regional Flood Control District  
97 East Congress, Tucson, AZ 85701

The project was prepared by:

Dave Stewart, EIT, Civil Engineering Assistant.  
Pima County Regional Flood Control District  
97 East Congress, Tucson, AZ 85701

### ***1.3 Project Location***

The study reach of the Del Cerro Wash (DCR) contains Federal Emergency Management Agency (FEMA)-designated “Zone A” flood-hazard areas, as depicted on FIRM Map Panel Numbers 04019C1616K (February 8, 1999). The existing “Zone A” flood hazard area is not consistent with current topographic data. The objective of the TDN and LOMR submission is to provide regulatory discharge rates and floodplain limits along the Del Cerro Wash using better topographic, hydrologic, and hydraulic data.

The study reach of the DCR is located primarily west of N. Silverbell Rd. in Sections 17, 18, 19, and 20 of Township 13 South, Range 13 East in Pima County, Arizona (Fig. 1).

### ***1.4 Hydrologic and Hydraulic Methods***

The hydrologic analysis was performed to determine proposed regulatory discharge rates at concentration points along the DCR using the U.S. Army Corps of Engineers Hydrologic Modeling System (HEC-HMS). The proposed regulatory discharges are flow rates that have a 1-percent chance of being equaled or exceeded each year (“100-year” discharge rates). Hydraulic analysis was performed to delineate floodplain limits along the study reach of the Del Cerro Wash using the Hydrologic Engineering Center’s River Analysis System.

No duplicate effective model is being provided because the proposed map revision represents a complete replacement of the Del Cerro wash based on better topographical data and revised hydrology. The revised hydrology is based on newer rainfall values from NOAA Atlas 14.

The annotated FIRM panel data was generated digitally. All shape files for the Flood Hazard Zones, Cross Sections, LOMR boundary boxes came from FEMA digital data. The digitally produced annotated FIRM panels were generated based on discussions between Terry Hendricks, Chief Hydrologist at Pima County Flood Control, and Joe Kuechenmeister, MT-2 Revisions Manager at Michael Baker Jr, Inc., about the problems with pasting paper LOMRs

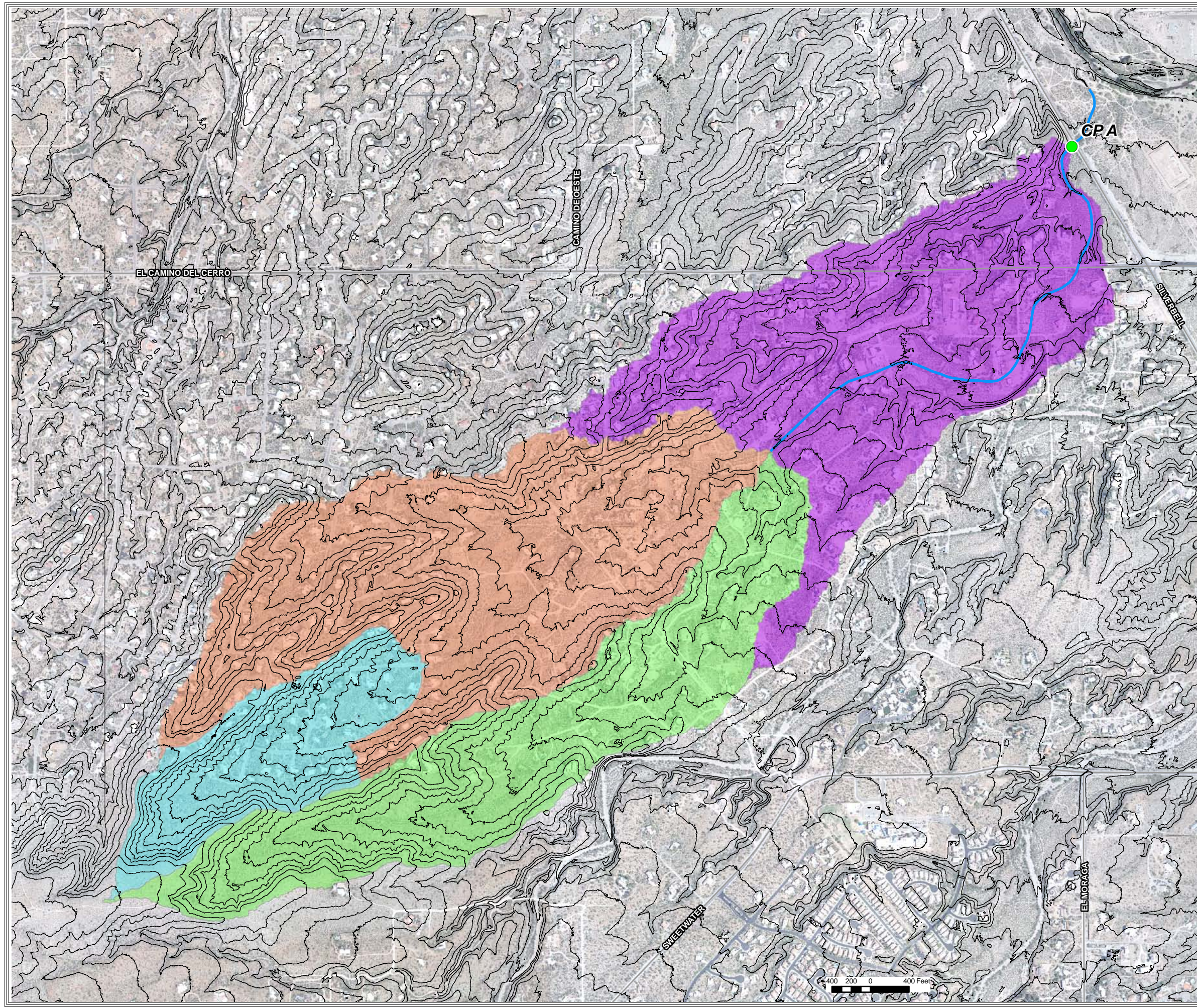
### ***1.4 Acknowledgments***

This study relied on assistance of RFCD staff, who were integral to the development of the models and maps.

### ***1.5 Study Results***

The regulatory discharge for floodplain mapping was calculated at Silverbell Rd. for the Del Cerro Wash (DCR A). The 100-yr discharge of the Del Cerro Wash at N. Silverbell Rd. (DCR A) was found to be 1182.0 cfs with a drainage area of 1.232 mi<sup>2</sup>.

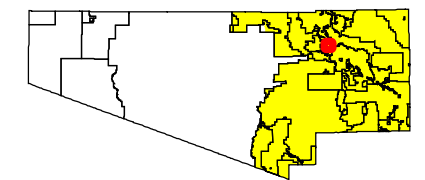
**Figure 1.1  
Watershed Map  
Del Cerro Wash**



- Del Cerro CP
- 10ft Contour
- Del Cerro Stream
- Del Cerro subbasin**
- DCR A
- DCR B
- DCR C
- DCR D

Aerial Photo: 2008 Pima Association of Governments

Pima County Index Map



Index Map Scale 1:5,250,000

The information depicted on this display is the result of digital analysis performed on a variety of databases provided and maintained by several governmental agencies. The accuracy of the information presented is limited to the collective accuracy of these databases on the date of the analysis. The Pima County Regional Flood Control Department makes no claims regarding the accuracy of the information depicted herein.

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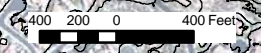
Pima County Regional Flood Control District



Scale 1:1000

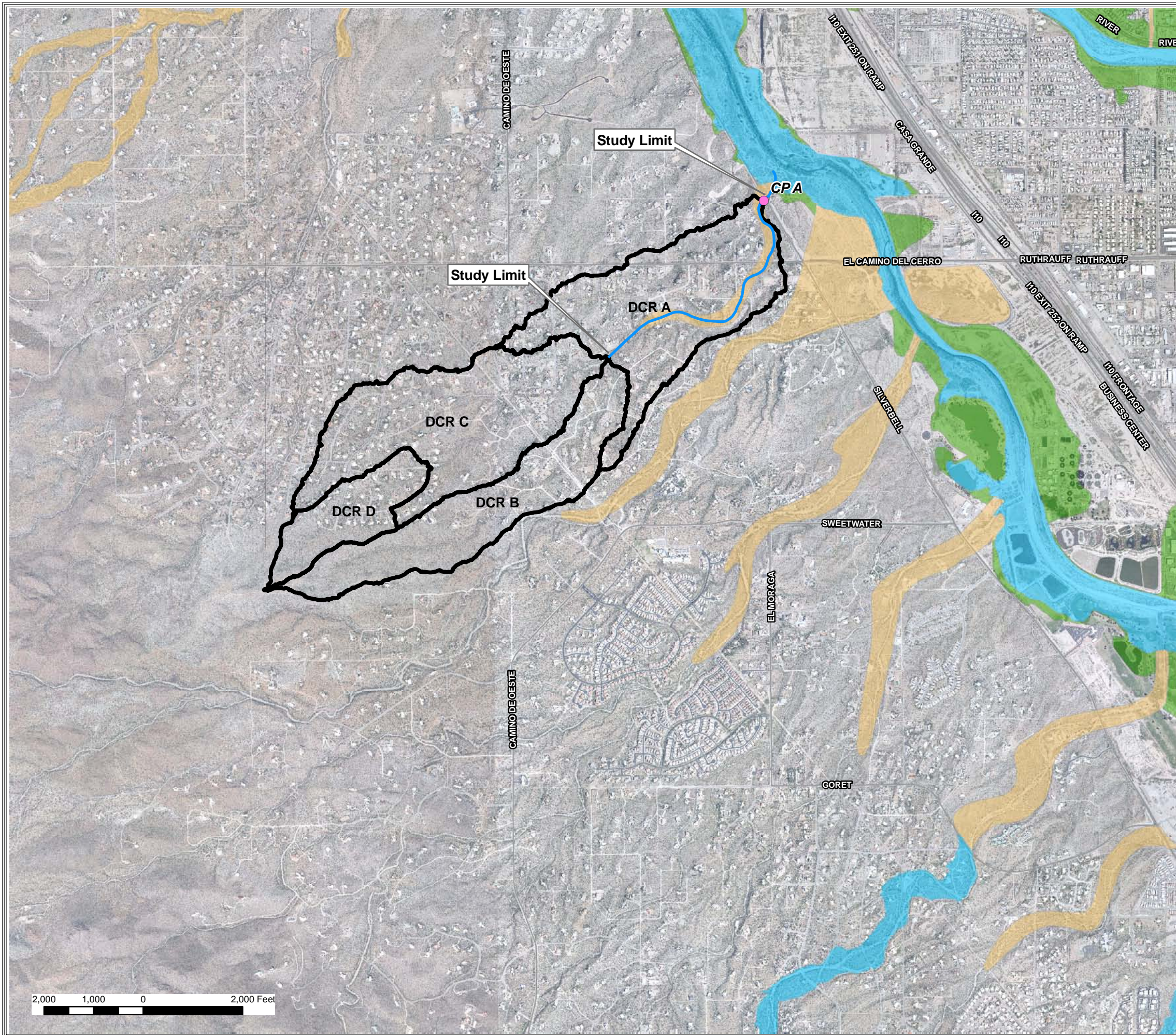


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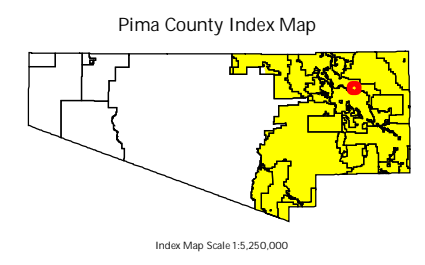


**Figure 1.2  
Study Limit  
Del Cerro Wash**



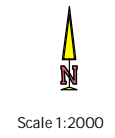
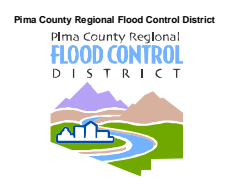
- Del Cerro CP
- Del Cerro Stream
- Del Cerro subbasins
- Existing FEMA Floodplain**
- ZONE A
- ZONE AE
- ZONE X - SHADED

Aerial Photo: 2008 Pima Association of Governments

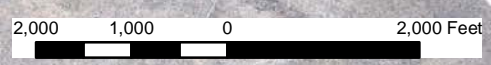


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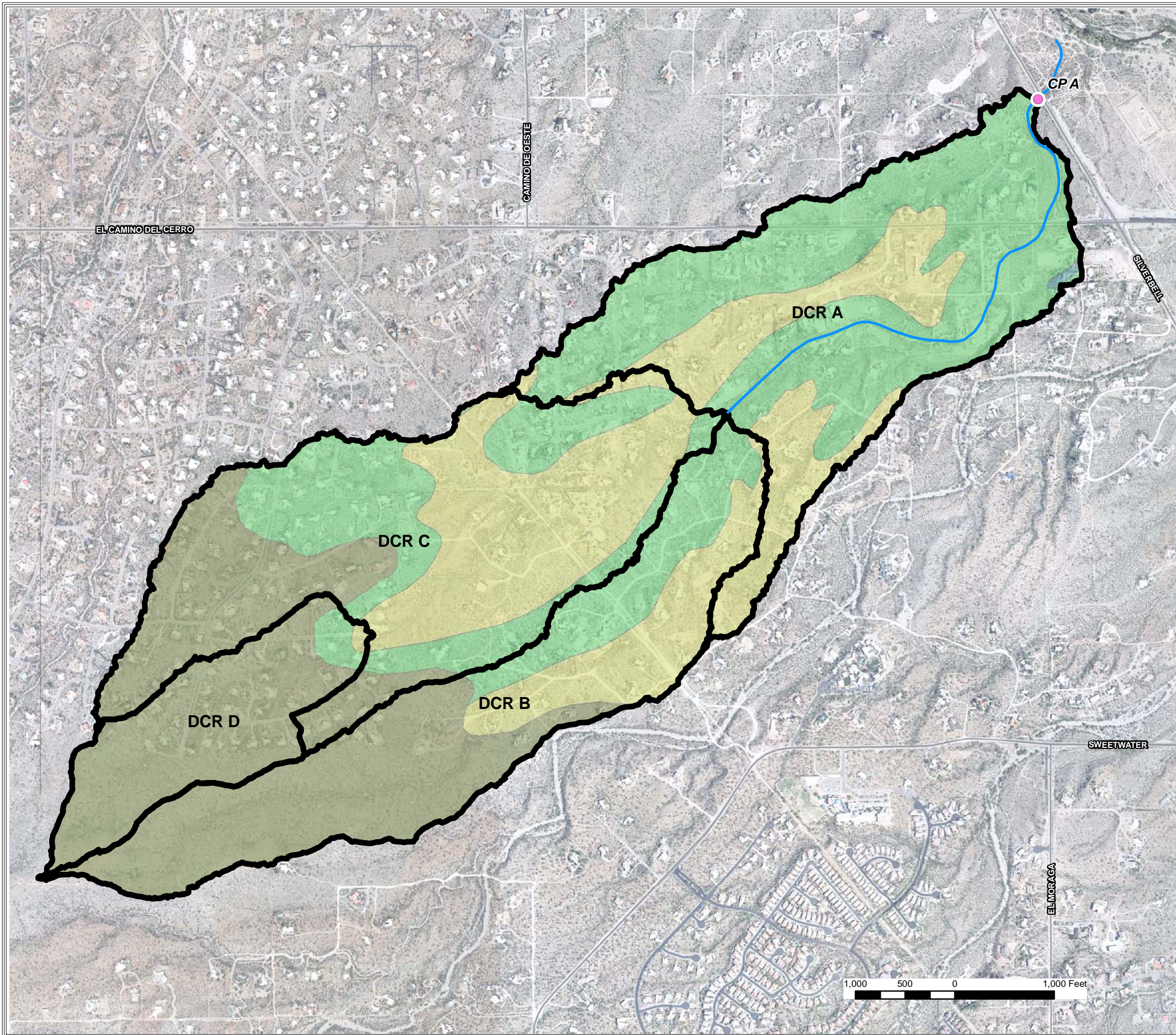
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**Figure 1.3  
Soil Classification  
Del Cerro Wash**

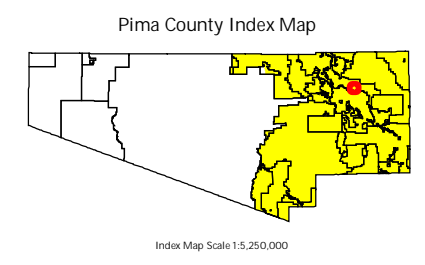


-  Del Cerro CP
-  Del Cerro Stream
-  Del Cerro subbasins

**Del Cerro Soil Classification**

-  Soil Group: B (100%), ANTHONY FINE SANDY LOAM, 0 TO 3 PERCENT SLOPES
-  Soil Group: B (100%), PINALENO VERY COBBLY SANDY LOAM, 1 TO 8 PERCENT SLOPES
-  Soil Group: B (100%), PINALENO-STAGECOACH COMPLEX, 5 TO 16 PERCENT SLOPES
-  Soil Group: B (82%) C (18%), PINALENO-STAGECOACH-PALOS VERDES COMPLEX, 10 TO 35 PERCENT SLOPES


Aerial Photo: 2008 Pima Association of Governments



The information depicted on this display is the result of digital analysis performed on a variety of databases provided and maintained by several governmental agencies. The accuracy of the information presented is limited to the collective accuracy of these databases on the date of the analysis. The Pima County Regional Flood Control Department makes no claims regarding the accuracy of the information depicted herein.

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## **Section 2 FEMA Forms**

### **2.1 Study Documentation Abstract for FEMA submittals**

**2.1.1 Date Study Accepted:** \_\_\_\_\_

**2.1.2 Study Contractor:**

Planning and Development Division,  
Pima County Regional Flood Control District  
97 East Congress, Tucson, AZ 85701  
(520) 243-1800

Prepared by Dave Stewart, Civil Engineering Assistant.

**2.1.3 Local Technical Reviewer:**

Terry Hendricks, C.F.M, Chief Hydrologist  
Planning and Development Division,  
Pima County Regional Flood Control District  
97 East Congress, Tucson, AZ 85701  
(520) 243-1800

**2.1.4 Reach Description**

The study reach of the Del Cerro Wash (DCR) is located within a Federal Emergency Management Agency (FEMA)-designated “Zone A” flood-hazard area, as depicted on FIRM Map Panel Numbers 04019C1616K (February 8, 1999). The study reach of the Del Cerro Wash is located primarily west of N. Silverbell Rd. in Sections 17, 18, 19, and 20 of Township 13 South, Range 13 East in Pima County, Arizona (Fig. 1).

The study reaches of the DCR contain coarse sand and cobble beds. Small shrubs and trees grow in some areas of the channel bed, and desert brush covers the overbanks.

**2.1.5 USGS Quad Sheets**

The study area is better described by the FEMA maps, which are referenced in Section 2.1.7.

### **2.1.6 Unique Conditions and Problems**

There were no unique conditions or problems.

### **2.1.7 Coordination of Peak Discharges**

The 100-year regulatory discharge rate at the concentration point was computed using HEC-HMS, assuming no base flow in the watersheds and no transmission loss within the reaches. All reaches were modeled with HEC-RAS. The discharge rates were acceptable per Suzanne Shields, Director of the Pima County Regional Flood Control District, and Andrew Dinauer, Engineering Administrator of the City of Tucson.

### **2.2 FEMA Forms**

The FEMA MT-2 forms are included at the end of this TDN.

## **Section 3 Survey and Mapping Information**

### **3.1 Field Survey Information**

No field survey was performed for this study.

### **3.2 Mapping**

The topographic data for the hydrology was obtained using 2008 Pima Associations of Governments (PAG) Light Detection and Ranging (LiDAR) data in Geo-RAS and ArcGIS. A raster was created from the 2008 LiDAR data with 5' cells and used with Geo-RAS.

For the hydraulic analysis, the triangular irregular network (TIN) developed by HDR in the Silverbell Road, Grant Road to Ina Road Design Concept Report (2009) was used. The TIN was developed from 2005 Lidar and supplemented with field survey methods that provide equal or better precision and accuracy to the Lidar data (Appendix C).

The following data was used in this TDN;

- The aerial photo: 2008 PAG aerial photo

- Projection: UTM, Zone 12

- Units: International feet

- The contour interval of the topographic map is 2 feet.

- Vertical Datum: NAVD 1988

## **Section 4 Hydrology**

### ***4.1 Method Description***

The 100-year peak discharges for the Del Cerro Wash were modeled using the U.S. Army Corps of Engineers Computer Hydrologic Modeling System, (HEC-HMS) version 3.2.

The HEC-HMS model requires parameters for rainfall, topography, soil, vegetation, and channel characteristics to determine runoff volume and peak discharge. Those parameters were determined according to the Pima County Regional Flood Control District Technical Policy 018 (Tech-018). Tech-018 is included in Appendix A. The HEC-HMS model is included in Appendix D.

### ***4.2 Parameter Estimation***

#### **4.2.1 Drainage Area**

The topographic data was obtained from a 5' cell raster created from 2008 PAG LiDAR data. ArcGIS was used to delineate watersheds from the raster and determine the drainage areas of each sub-basin. The composite watershed map is included in Figure 1.1.

#### **4.2.2 Watershed Work Map**

Four sub-basins were delineated for the Del Cerro Wash basin. The 100-year peak discharge was calculated for DCR A and used in the HEC-RAS hydraulic analysis for the floodplain maps.

#### **4.2.3 Gage Data**

No gage data were used in this TDN.

#### **4.2.4 Statistical Parameters**

No data record was available for the Del Cerro Wash and therefore no Bulletin 17B analysis was used for this TDN.

#### **4.2.5 Precipitation**

According to Tech-018, the design storm should be used that produces the higher discharge between the 100-yr 3-hour SCS Type II distribution and the 100-yr 24-hr SCS Type I distribution. The 100-yr 3-hour SCS Type II distribution was found to produce the higher discharge on the Del Cerro Wash.

NOAA Atlas 14, upper 90% confidence interval precipitation frequency estimate values (NOAA 14 rainfall) were used to determine 3-hour and 24-hour point rainfall depths for the watershed. The point rainfall depth for the 3-hour storm was obtained for the coordinates of the watershed centroid. An areal reduction factor was applied to watersheds larger than 1 square mile as noted in Tech-018.

#### 4.2.6 Physical Parameters

The physical parameters for the sub-basins and reaches of the HEC-HMS model are summarized in Tables 4.1 and 4.2. As mentioned in Section 4.1, all the methods and parameters were determined based on Tech-018. Table 4.1 summarizes the method used for the HEC-HMS analysis.

**Table 4. 1.** Methods used for the HEC-HMS analysis

	<b>Selected Method</b>
Rainfall Depth	NOAA 14, upper 90% Confidence Interval
Rainfall Distribution	3-hr SCS Type II Storm
Rainfall Loss	SCS Curve number
Time of Concentration	SCS Segmental Method
Transform	SCS Unit Hydrograph
Routing	Modified-Puls

The SCS Curve Number (CN) method was utilized as a rainfall loss method in the HEC-HMS model. The CN was determined using the Curve Number tables and Hydrologic Soils Group maps associated with the PC Hydro User Guide (Arroyo Engineering, 2007). The CN was not adjusted for rainfall intensity or antecedent moisture conditions.

The SCS Unit Hydrograph method was used as a transform method. Impervious cover was determined using 2008 PAG aerial photographs. The combination of the kinematic wave time of concentration method and the U.S. Natural Resources Conservation Service (NRCS) segmented Time of Concentration (Tc) calculation (USDA-NRCS, 1986) was used to determine Tc, based on the recommendation on Tech-018. The Tc was calculated by summing the travel time for sheet flow, shallow concentrated flow and channel flow. The Tc for sheet flow was estimated using the kinematic wave equation. The Manning’s roughness coefficient for sheet flow was obtained using Table 3-1 in Technical Release 55, Urban Hydrology for Small Watersheds (USDA-NRCS, 1986). The channel velocity was calculated using Manning’s equation. The Tc calculations are included in Appendix D.

**Table 4. 2.** Physical Parameters for the Sub-Basins.

<b>Sub-basin</b>	<b>Area (sq mi)</b>	<b>CN</b>	<b>Impervious Area (%)</b>	<b>Vegetation Cover (%)</b>	<b>Lag Time (min)</b>
DCR A	0.393	85.7	16.7	20	18.6
DCR B	0.295	85.5	12.9	20	26.5
DCR C	0.414	85.9	16.5	20	18.3
DCR D	0.129	86.4	16.8	20	13.9

Runoff from sub-basins was routed using the Modified-Puls method. A storage discharge table for the channel routing was developed using the cross sections and slopes derived from HEC-HMS. The number of sub-reaches was calculated using the following method:

$$V_w = 1.5 * V_{ave} \dots \dots \dots eq.1$$

$$K = \frac{L}{V_w} \dots \dots \dots eq.2$$

Therefore,

$$N = \frac{K}{\Delta t} \dots \dots \dots eq.3$$

where  $V_{ave}$  is the average flow velocity,  $L$  is the reach length,  $V_w$  is the velocity of the flood wave (a conversion factor of 1.5 is used for natural channels),  $K$  is the hydrograph travel time,  $\Delta t$  is the time interval for computations in the model, and  $N$  is the number of steps in the reach routing. Eq.4 was obtained from eq.1, 2, and 3. The detail of the calculation of the number of sub-reach is included in Appendix D.

### **4.3 Problems Encountered During the Study**

#### **4.3.1 Special Problems and Solutions**

There were no problems with the hydrologic modeling.

#### **4.3.2 Modeling Warning and Error Messages**

The time interval of the rainfall data used in this study is 5 minutes, while the simulation time interval is 1 minute. The HEC-HMS model interpolated the 5-minute time interval of the rainfall data to 1-minute time interval.

The following warnings were produced in HEC-HMS;

- The “3-hr SCS Type II” gage with data interval of 5 minutes was interpolated to a simulation time interval of 1 minute.

#### 4.4 Calibration

No calibration was conducted in this study.

#### 4.5 Final Results

##### 4.5.1 Hydrologic Analysis Results

The 100-year peak discharges at the concentration points along the DCR were determined using HEC-HMS. The results are summarized in Tables 4.3 and 4.4.

**Table 4.3.** Summary of the Hydrologic Analysis Results for Sub-Basins.

Sub-basin	Area (Sq mi)	Rainfall Depth (in)	Runoff Volume (in)	Peak Discharge (cfs)
DCR A	0.393	2.98	1.62	576.8
DCR B	0.295	2.98	1.61	335.7
DCR C	0.414	2.98	1.64	620.1
DCR D	0.129	2.98	1.68	235.0

**Table 4. 4.** Summary of the Hydrologic Analysis Results at the Concentration Points.

CP	Location	Drainage Area (mi <sup>2</sup> )	Rainfall (in)	Runoff Volume (in)	Qp (cfs)	Time to Peak
DCR A	Del Cerro Wash at Silverbell Rd.	1.23	2.98	1.62	1182.0	1:43

##### 4.5.2 Verification of Results

The modeled 100-yr peak discharge was found to be lower than the Regional Regression Equation 13 (Thomas et al., 1997) peak discharge (Table 5).

**Table 4.5.** Comparison of modeled peak discharge to the Regional Regression Equation 13 peak discharge.

CP	Location	Area (mi <sup>2</sup> )	Q <sub>p100</sub> <sup>HMS</sup> (cfs)	Q <sub>p100</sub> <sup>RRE</sup> (cfs)
DCR A	Del Cerro Wash at Silverbell Rd.	1.23	1182.0	1443.4



## **Section 5 Hydraulics**

### ***5.1 Method Description***

The hydraulic modeling for the DCR was performed using HEC-RAS, Version 4.0, HEC-GeoRAS, Version 4.1.1, and ArcGIS, Version 9.2.

The topographic data was obtained using a triangular irregular network (TIN) developed by HDR from 2005 Lidar and supplemented with survey (Appendix C). The locations of the stream centerlines were determined using ArcGIS with the contour lines from the topographic data and 2008 PAG aerial photos.

The physical attributes of the wash were digitized in ArcGIS using the HEC-GeoRAS extension and then exported to HEC-RAS to create geospatially-referenced geometric data (cross sections, reach lengths). Other parameters for the steady-state analysis, such as the Manning's n-values, obstructions, and ineffective flow areas were manually entered into HEC-RAS. The hydraulic data obtained from HEC-RAS were exported to ArcGIS to delineate the floodplain in the study area.

The hydraulic analysis was performed in the area currently mapped as FEMA Zone A. A steady flow analysis was performed to determine 100-year water surface elevations in the study area by using HEC-RAS. The HEC-RAS data and shape files (contour lines, flow path, cross section lines, study watersheds, concentration points, sub-watersheds, hydrologic soil groups, proposed floodplain limit) used in the analysis are included in Appendix D with an attached CD.

A normal depth downstream boundary condition was assumed using the average bed slope calculated from the two most downstream cross sections.

### ***5.2 Work Study Maps***

The work study map for the Del Cerro Wash is included in Exhibit 1.

### ***5.3 Parameter Estimation***

#### ***5.3.1 Roughness Coefficients***

Manning's n values were determined in the field and using 2008 PAG aerial photo based on USGS publications for Manning's n values in southern Arizona (Phillips and Tadayon, 2006). The channel bank stations were assigned in HEC-RAS to be wide enough to provide an accurate hydraulic radius for the channel. Therefore, overbank vegetation outside of the sand bed is included in the HEC-RAS channel flow path for the 100-yr flood and is included in the Manning's n value assigned for the channel.

A Manning's n value of 0.045 was assigned for the sand bed and channel due to the dense vegetation in the flow path, and a value of 0.060 was assigned for the overbank areas due to the desert brush and shallow flow depth (Figures 5.1 and 5.2).



**Figure 5.1.** Del Cerro Wash Photo 1. The center of the flow path of the Del Cerro Wash immediately upstream of Silverbell Rd.



**Figure 5.2.** Del Cerro Wash Photo 2. The center of the flow path of the Del Cerro Wash immediately downstream of Silverbell Rd.

### **5.3.2 Expansion and Contraction Coefficients**

The Del Cerro Wash is assumed to have relatively gradual transitions, and the default values for the expansion and contraction coefficient of 0.30 and 0.10 were used respectively.

### **5.4 Cross-Section Description**

Cross-section locations were determined primarily based on the channel topography from the TIN and 2008 PAG LiDAR data. Cross sections were placed at a spacing of approximately 100 ft or less. The cross-section lines were drawn to be perpendicular to flow paths in ArcGIS.

### **5.5 Modeling Consideration**

#### **5.5.1 Hydraulic Jump and Drop Analysis**

No hydraulic jumps or hydraulic drops were modeled in this study.

#### **5.5.2. Bridges and Culverts**

There are no bridges located in the floodplain study reach of the Del Cerro Wash.

#### **5.5.3 Levees and Dikes**

There are no levees or dikes located within the study limit.

#### **5.5.4 Island and Flow Splits**

No islands or split flows were modeled in the study.

#### **5.5.5 Ineffective Flow Areas**

The ineffective flow areas were used in situations where:

- The floodplain areas are not hydraulically connected
- There is a contraction or expansion of the effective flow area either from an obstruction in the flowpath or from the channel banks.

## ***5.6 Floodway Modeling***

No floodway modeling was performed in this study.

## ***5.7 Problems Encountered***

### **5.7.1 Special Problems and Solutions**

There were no special problems encountered in this study.

### **5.7.2 Model Warnings and Errors**

The FEMA guidelines require hydraulic models to simulate subcritical flow conditions. The HEC-RAS modeling produced warnings at some cross sections stating that:

- The energy equation could not be balanced within the specified number of iterations
- The energy loss was greater than 1.0 ft between cross sections
- The conveyance ratio is less than 0.7 or greater than 1.4
- Multiple critical depths were found
- Divided flow was computed and
- The program defaulted to critical depth when the water surface was calculated below critical depth at some locations.”

A summary of warnings is available in Appendix E.

## ***5.8 Calibration***

The model was not calibrated in this study.

## ***5.9 Final Results***

### **5.9.1 Hydraulic Analysis Results**

The HEC-RAS modeling results are summarized in Appendix E.

### **5.9.2 Verification of Results**

The floodplain limit produced in this LOMR study was compared to the existing FEMA floodplain limit. The proposed floodplain limit is reasonable based on the existing FEMA floodplain limit and provides greater accuracy based on contour lines from the updated topographic data.

## **Section 6 Erosion and Sediment Transport**

No erosion or sediment transport analysis was conducted in this study.

## **Section 7 Draft FIS Report Data**

### ***7.1 Summary of Discharges***

The peak discharge at DCR A was calculated as 1182.0 cfs.

### ***7.2 Floodway Data***

Not applicable.

### ***7.3 Annotated Flood Insurance Rate Map***

An annotated Flood Insurance Rate Map (FIRM) is included in Exhibit 2.

### ***7.4 Flood Profiles***

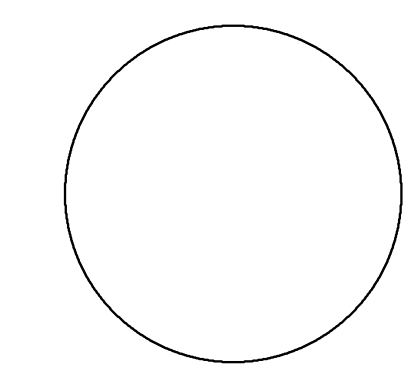
Flood profiles are included in Appendix E.

# Exhibit 1 100-year Floodplain with cross sections Del Cerro Wash

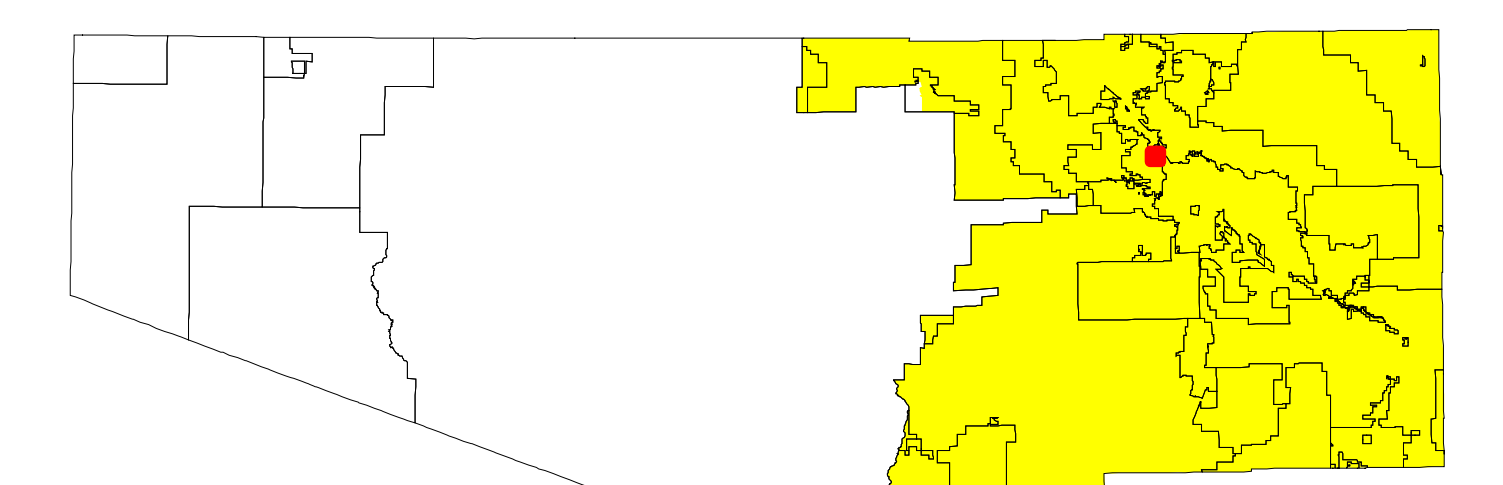


- Discharge Point
- Cross Sections
- Stream
- Contours 2 foot
- Contour 10 foot
- Existing Zone AE
- Proposed 100yr Floodplain

Aerial Photo: 2008 Pima Association of Governments  
 Topo: 2005 Pima Association of Governments  
 with Supplemental Survey  
 Vertical Datum: NAVD 1988



Pima County Index Map



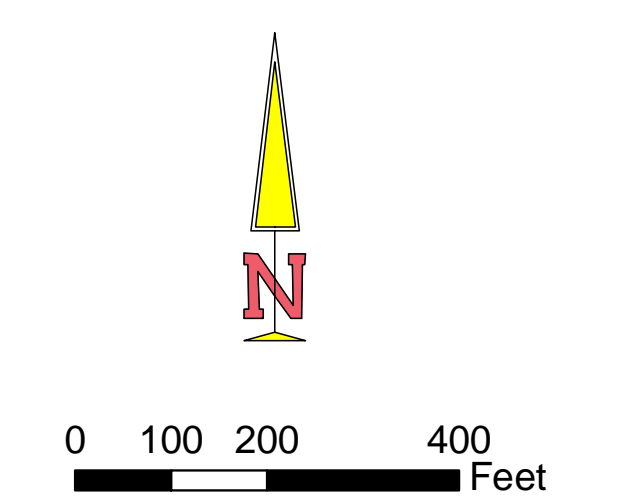
Index Map Scale 1:1,500,000

The information depicted on this display is the result of digital analyses performed on a variety of databases provided and maintained by several governmental agencies. The accuracy of the information presented is limited to the collective accuracy of these databases on the date of the analysis. The Pima County Department of Transportation Technical Services Division makes no claims regarding the accuracy of the information depicted herein.

This product is subject to the Department of Transportation Technical Services Division's Disclaimer and Use Restrictions.  
**Pima County Regional Flood Control District**

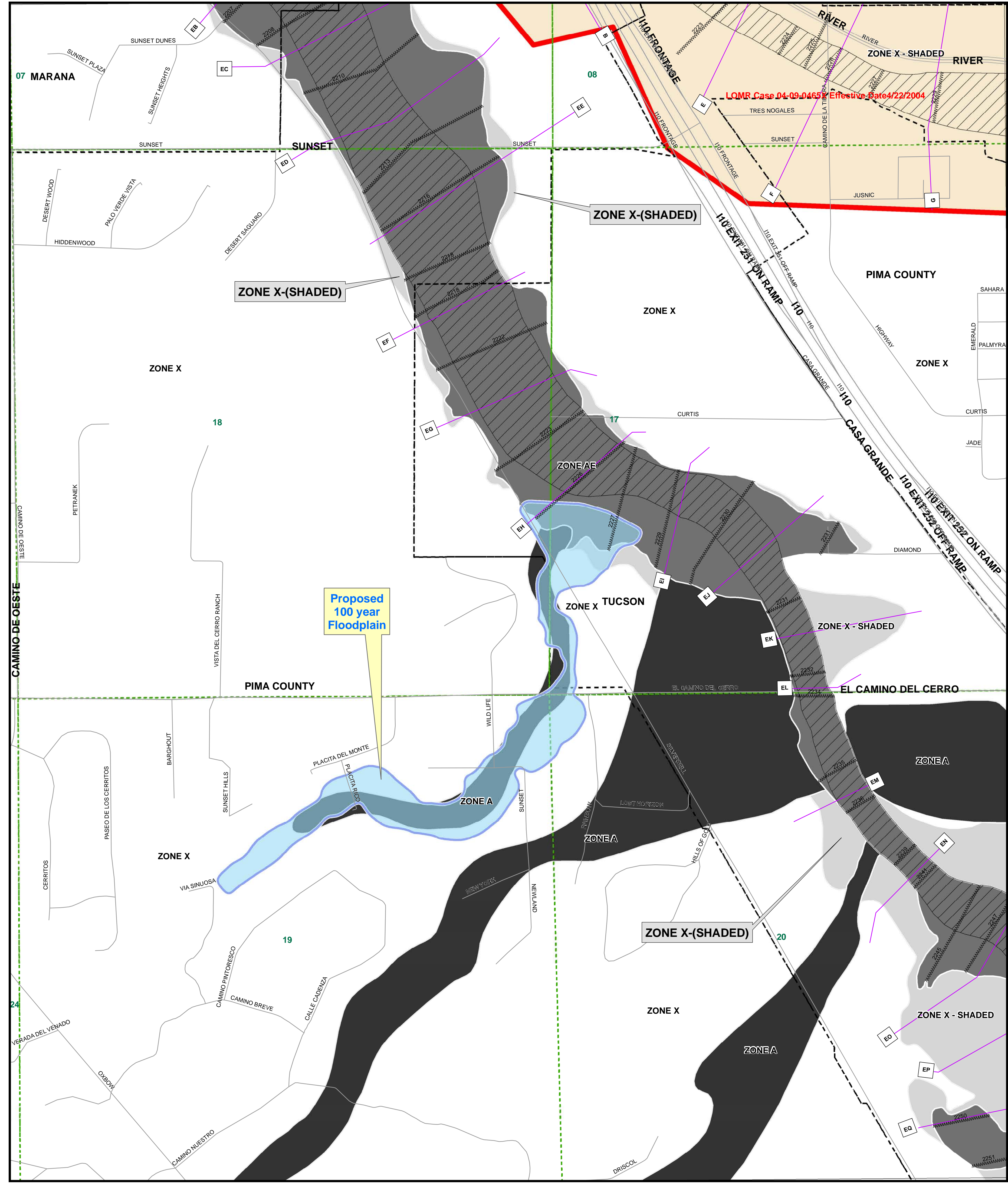


Pima County Regional Flood Control  
 97 East Congress Street - 3rd Floor  
 Tucson, Arizona 85701-1207  
 (520)243-1800 - FAX (520)243-1821  
<http://www.rfcd.pima.gov>

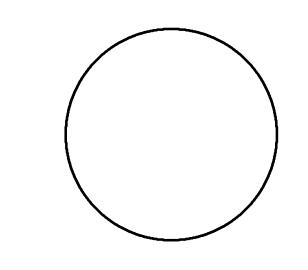


09/2010

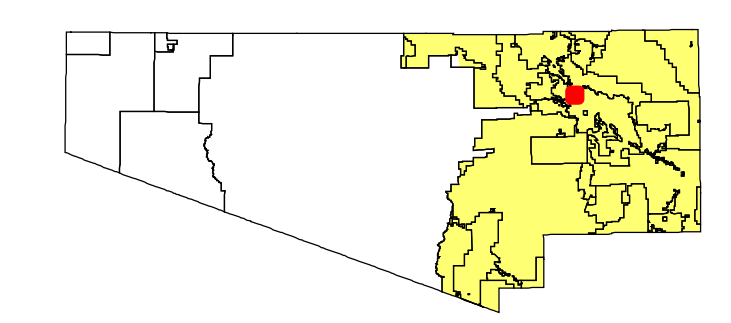
## Exhibit 2 Annotated Flood Insurance Rate Map 04019C1616 K Del Cerro Wash



- Streets
  - FIRM X-Sections
  - ~~~~~ Base Flood Elevations
  - █ Proposed Floodplain
  - █ LOMR Case Studies
  - █ FIRM - Flood Insurance Rate Map
  - ▨ Floodways
  - Sections
  - Jurisdictions
- Existing Floodplain Zone**
- █ A
  - █ AE
  - █ AO
  - █ X
  - █ X - (SHADED)

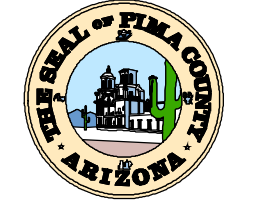
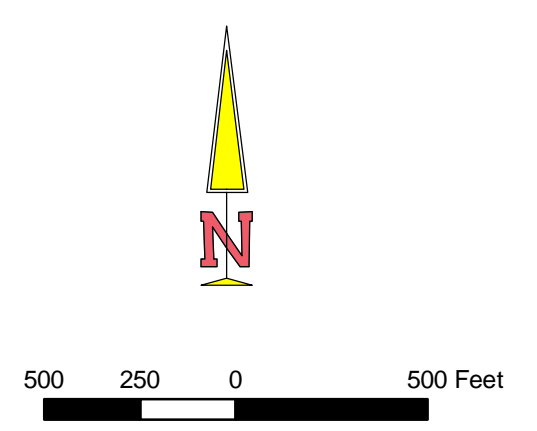


Pima County Index Map



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 Tucson, Arizona 85701-1207  
 (520) 241-1800 - FAX (520) 241-1821  
<http://www.fcd.pima.gov>

## ***A.1 Data Collection Summary***

Aldridge, B. and J. Garrett. 1973. Roughness Coefficients for Stream Channels in Arizona. US Department of the Interior Geological Survey. Tucson, AZ.

Arizona Department of Water Resources, Flood Mitigation Section  
“Requirements for Flood Study Technical Documentation” SS1-97, November 1997

National Weather Service. 1984. Depth-Area Ratios in the Semi-Arid Southwest United States, NOAA Technical Memorandum NWS Hydro-40

Phillips, J., and S. Tadayon. 2006. Selection of Manning’s roughness coefficient for natural and constructed vegetated and non-vegetated channels, and vegetation maintenance plan guidelines for vegetated channels in central Arizona: U.S. Geological Survey Scientific Investigations Report 2006–5108, 41 p.

Phillips, J., and T. Ingersoll. 1998. Verification of Roughness Coefficients for Selected Natural and Constructed Stream Channels in Arizona. U.S. Geological Survey Professional Paper 1584.

Pima County Regional Flood Control District  
“Pima County Mapguide Map”, 2008

U.S. Army Corps of Engineers (COE). 2001. HEC-RAS, River Analysis System, Hydraulic Reference Manual, CPD-69, Hydraulic Engineering Center, Davis, CA.

U.S. Army Corps of Engineers (COE). 2003. Geospatial Hydrologic Modeling Extension HEC-GeoHMS, (v 1.1) CPD-77, Hydraulic Engineering Center, Davis, CA.

U.S. Army Corps of Engineers (COE). 2006. HEC-HMS, Hydrologic Modeling System User’s Manual, (v. 3.1.0) CPD-74A, Hydraulic Engineering Center, Davis, CA.

U.S. Department of Agriculture Natural Resources Conservation Service (NRCS), 1986. Urban Hydrology for Small Watersheds, Technical Release 55. Washington, DC.



## ***A 2. Referenced Documents***

Eychaner, J.H., 1984. Estimation of magnitude and frequency of floods in Pima County, Arizona, with comparisons of alternative methods: U.S. Geological Survey Water-Resources Investigations Report 84-4142, 69 p.

Haan, C.T., Barfield, B.J., Hayes, J.C. 1994. Design Hydrology and Sedimentology for Small Catchments, Academic Press.

Thomas, B.E., H.W. Hjalmanson, and S.D. Waltemeyer. 1997. Methods for Estimating Magnitude and Frequency of Floods in the Southwestern United States. USGS Water Supply Paper 2433. 195 p.

U.S. Department of Agriculture Natural Resources Conservation Service (NRCS), 1986. Urban Hydrology for Small Watersheds, Technical Release 55. Washington, DC.

## **Appendix B General Documentation and Correspondence**

**U.S. DEPARTMENT OF HOMELAND SECURITY - FEDERAL EMERGENCY MANAGEMENT AGENCY  
OVERVIEW & CONCURRENCE FORM**

*O.M.B No. 1660-0016  
Expires: 12/31/2010*

**PAPERWORK BURDEN DISCLOSURE NOTICE**

Public reporting burden for this form is estimated to average 1 hour per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing, reviewing, and submitting the form. You are not required to respond to this collection of information unless a valid OMB control number appears in the upper right corner of this form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden to: Information Collections Management, U.S. Department of Homeland Security, Federal Emergency Management Agency, 500 C Street, SW, Washington DC 20472, Paperwork Reduction Project (1660-0016). Submission of the form is required to obtain or retain benefits under the National Flood Insurance Program. **Please do not send your completed survey to the above address.**

**A. REQUESTED RESPONSE FROM DHS-FEMA**

This request is for a (check one):

- CLOMR: A letter from DHS-FEMA commenting on whether a proposed project, if built as proposed, would justify a map revision, or proposed hydrology changes (See 44 CFR Ch. 1, Parts 60, 65 & 72).
- LOMR: A letter from DHS-FEMA officially revising the current NFIP map to show the changes to floodplains, regulatory floodway or flood elevations. (See 44 CFR Ch. 1, Parts 60, 65 & 72)

**B. OVERVIEW**

1. The NFIP map panel(s) affected for all impacted communities is (are):

Community No.	Community Name	State	Map No.	Panel No.	Effective Date
Ex: 480301	City of Katy	TX	480301	0005D	02/08/83
480287	Harris County	TX	48201C	0220G	09/28/90
040073	Pima County	AZ	04019C	1616K	02/08/99
040078	City of Tucson	AZ	04019C	1616K	02/08/99

2. a. Flooding Source: Del Cerro Wash

- b. Types of Flooding:  Riverine     Coastal     Shallow Flooding (e.g., Zones AO and AH)  
 Alluvial fan     Lakes     Other (Attach Description)

3. Project Name/Identifier: Del Cerro Wash

4. FEMA zone designations affected: A (choices: A, AH, AO, A1-A30, A99, AE, AR, V, V1-V30, VE, B, C, D, X)

5. Basis for Request and Type of Revision:

a. The basis for this revision request is (check all that apply)

- Physical Change     Improved Methodology/Data     Regulatory Floodway Revision     Base Map Changes  
 Coastal Analysis     Hydraulic Analysis     Hydrologic Analysis     Corrections  
 Weir-Dam Changes     Levee Certification     Alluvial Fan Analysis     Natural Changes  
 New Topographic Data     Other (Attach Description)

Note: A photograph and narrative description of the area of concern is not required, but is very helpful during review.

b. The area of revision encompasses the following structures (check all that apply)

- Structures:     Channelization     Levee/Floodwall     Bridge/Culvert  
 Dam     Fill     Other (Attach Description)

**C. REVIEW FEE**

Has the review fee for the appropriate request category been included?  Yes Fee amount: \$ \_\_\_\_\_  
 No, Attach Explanation

Please see the DHS-FEMA Web site at [http://www.fema.gov/plan/prevent/fhm/frm\\_fees.shtm](http://www.fema.gov/plan/prevent/fhm/frm_fees.shtm) for Fee Amounts and Exemptions.

**D. SIGNATURE**

All documents submitted in support of this request are correct to the best of my knowledge. I understand that any false statement may be punishable by fine or imprisonment under Title 18 of the United States Code, Section 1001.

Name: Dave Stewart, E.I.T.		Company: Pima County Regional Flood Control	
Mailing Address: 97 E. Congress, Tucson AZ, 85701		Daytime Telephone No.: 520 243 1800	Fax No.: 520 243-1821
		E-Mail Address: Dave.Stewart@rfgcd.pima.gov	
Signature of Requester (required): <i>Dave Stewart</i>		Date: <i>05/17/2010</i>	

As the community official responsible for floodplain management, I hereby acknowledge that we have received and reviewed this Letter of Map Revision (LOMR) or conditional LOMR request. Based upon the community's review, we find the completed or proposed project meets or is designed to meet all of the community floodplain management requirements, including the requirement that no fill be placed in the regulatory floodway, and that all necessary Federal, State, and local permits have been, or in the case of a conditional LOMR, will be obtained. In addition, we have determined that the land and any existing or proposed structures to be removed from the SFHA are or will be reasonably safe from flooding as defined in 44CFR 65.2(c), and that we have available upon request by FEMA, all analyses and documentation used to make this determination.

Community Official's Name and Title: Andrew Dinauer, Engineering Administrator		Community Name: City of Tucson	
Mailing Address: P.O. Box 27210 Tucson, AZ 85726-7210		Daytime Telephone No.: 520-791-4251	Fax No.:
		E-Mail Address: adinaue1@ci.tucson.az.us	
Community Official's Signature (required): <i>[Signature]</i>		Date: <i>5/19/10</i>	

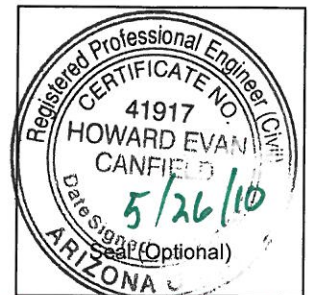
**CERTIFICATION BY REGISTERED PROFESSIONAL ENGINEER AND/OR LAND SURVEYOR**

This certification is to be signed and sealed by a licensed land surveyor, registered professional engineer, or architect authorized by law to certify elevation information data, hydrologic and hydraulic analysis, and any other supporting information as per NFIP regulations paragraph 65.2(b) and as described in the MT-2 Forms Instructions. All documents submitted in support of this request are correct to the best of my knowledge. I understand that any false statement may be punishable by fine or imprisonment under Title 18 of the United States Code, Section 1001.

Certifier's Name: Howard Evan Canfield	License No.: 41917	Expiration Date: 3/31/2011
Company Name: Pima County Regional Flood Control	Telephone No.: 520-243-1836	Fax No.:
Signature: <i>Howard Evan Canfield</i>		Date: <i>5/26/10</i>

Ensure the forms that are appropriate to your revision request are included in your submittal.

<u>Form Name and (Number)</u>	<u>Required if ...</u>
<input checked="" type="checkbox"/> Riverine Hydrology and Hydraulics Form (Form 2)	New or revised discharges or water-surface elevations
<input type="checkbox"/> Riverine Structures Form (Form 3)	Channel is modified, addition/revision of bridge/culverts, addition/revision of levee/floodwall, addition/revision of dam
<input type="checkbox"/> Coastal Analysis Form (Form 4)	New or revised coastal elevations
<input type="checkbox"/> Coastal Structures Form (Form 5)	Addition/revision of coastal structure
<input type="checkbox"/> Alluvial Fan Flooding Form (Form 6)	Flood control measures on alluvial fans



*Expires 3/31/10*

**U.S. DEPARTMENT OF HOMELAND SECURITY - FEDERAL EMERGENCY MANAGEMENT AGENCY  
OVERVIEW & CONCURRENCE FORM**

*O.M.B No. 1660-0016  
Expires: 12/31/2010*

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- LOMR: A letter from DHS-FEMA officially revising the current NFIP map to show the changes to floodplains, regulatory floodway or flood elevations. (See 44 CFR Ch. 1, Parts 60, 65 & 72)

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040073	Pima County	AZ	04019C	1616K	02/08/99
040078	City of Tucson	AZ	04019C	1616K	02/08/99

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- b. Types of Flooding:  Riverine     Coastal     Shallow Flooding (e.g., Zones AO and AH)  
 Alluvial fan     Lakes     Other (Attach Description)

3. Project Name/Identifier: Del Cerro Wash

4. FEMA zone designations affected: A (choices: A, AH, AO, A1-A30, A99, AE, AR, V, V1-V30, VE, B, C, D, X)

5. Basis for Request and Type of Revision:

a. The basis for this revision request is (check all that apply)

- Physical Change     Improved Methodology/Data     Regulatory Floodway Revision     Base Map Changes  
 Coastal Analysis     Hydraulic Analysis     Hydrologic Analysis     Corrections  
 Weir-Dam Changes     Levee Certification     Alluvial Fan Analysis     Natural Changes  
 New Topographic Data     Other (Attach Description)

Note: A photograph and narrative description of the area of concern is not required, but is very helpful during review.

b. The area of revision encompasses the following structures (check all that apply)

- Structures:     Channelization     Levee/Floodwall     Bridge/Culvert  
 Dam     Fill     Other (Attach Description)

**C. REVIEW FEE**

Has the review fee for the appropriate request category been included?  Yes Fee amount: \$ \_\_\_\_\_  
 No, Attach Explanation

Please see the DHS-FEMA Web site at [http://www.fema.gov/plan/prevent/fhm/frm\\_fees.shtm](http://www.fema.gov/plan/prevent/fhm/frm_fees.shtm) for Fee Amounts and Exemptions.

**D. SIGNATURE**

All documents submitted in support of this request are correct to the best of my knowledge. I understand that any false statement may be punishable by fine or imprisonment under Title 18 of the United States Code, Section 1001.

Name: Dave Stewart, E.I.T.	Company: Pima County Regional Flood Control	
Mailing Address: 97 E. Congress, Tucson AZ, 85701	Daytime Telephone No.: 520 243 1800	Fax No.: 520 243-1821
	E-Mail Address: Dave.Stewart@rfcd.pima.gov	
Signature of Requester (required): <i>Dave Stewart</i>	Date: <i>05/17/2010</i>	

As the community official responsible for floodplain management, I hereby acknowledge that we have received and reviewed this Letter of Map Revision (LOMR) or conditional LOMR request. Based upon the community's review, we find the completed or proposed project meets or is designed to meet all of the community floodplain management requirements, including the requirement that no fill be placed in the regulatory floodway, and that all necessary Federal, State, and local permits have been, or in the case of a conditional LOMR, will be obtained. In addition, we have determined that the land and any existing or proposed structures to be removed from the SFHA are or will be reasonably safe from flooding as defined in 44CFR 65.2(c), and that we have available upon request by FEMA, all analyses and documentation used to make this determination.

Community Official's Name and Title: Suzanne Shields, PE, Chief Engineer	Community Name: Pima County Flood Control	
Mailing Address: 97 E. Congress St. Tucson, AZ 85701	Daytime Telephone No.: 520-243-1800	Fax No.:
	E-Mail Address: Suzanne.Shields@rfcd.pima.gov	
Community Official's Signature (required): <i>Suzanne Shields</i>	Date: <i>5/26/2010</i>	

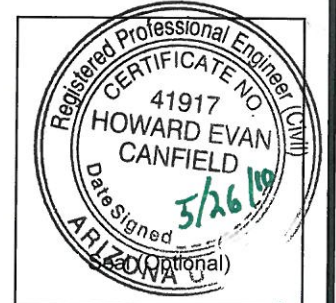
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Certifier's Name: Howard Evan Canfield	License No.: 41917	Expiration Date: 3/31/2011
Company Name: Pima County Regional Flood Control	Telephone No.: 520-243-1836	Fax No.:
Signature: <i>Howard Evan Canfield</i>	Date: <i>5/26/10</i>	

Ensure the forms that are appropriate to your revision request are included in your submittal.

Form Name and (Number)	Required if ...
<input checked="" type="checkbox"/> Riverine Hydrology and Hydraulics Form (Form 2)	New or revised discharges or water-surface elevations
<input type="checkbox"/> Riverine Structures Form (Form 3)	Channel is modified, addition/revision of bridge/culverts, addition/revision of levee/floodwall, addition/revision of dam
<input type="checkbox"/> Coastal Analysis Form (Form 4)	New or revised coastal elevations
<input type="checkbox"/> Coastal Structures Form (Form 5)	Addition/revision of coastal structure
<input type="checkbox"/> Alluvial Fan Flooding Form (Form 6)	Flood control measures on alluvial fans



*Expires 3/31/2011*

**PAPERWORK REDUCTION ACT**

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Flooding Source: Del Cerro Wash  
**Note:** Fill out one form for each flooding source studied

**A. HYDROLOGY**

1. Reason for New Hydrologic Analysis (check all that apply)

- Not revised (skip to section B)     
  No existing analysis     
  Improved data  
 Alternative methodology     
  Proposed Conditions (CLOMR)     
  Changed physical condition of watershed

2. Comparison of Representative 1%-Annual-Chance Discharges

Location	Drainage Area (Sq. Mi.)	Effective/FIS (cfs)	Revised (cfs)
Silverbell Rd.	1.23	N/A	1182.0
.		N/A	
.		N/A	

3. Methodology for New Hydrologic Analysis (check all that apply)

- Statistical Analysis of Gage Records     
  Precipitation/Runoff Model  
 Regional Regression Equations     
  Other (please attach description)

Please enclose all relevant models in digital format, maps, computations (including computation of parameters) and documentation to support the new analysis.

4. Review/Approval of Analysis

If your community requires a regional, state, or federal agency to review the hydrologic analysis, please attach evidence of approval/review.

5. Impacts of Sediment Transport on Hydrology

Was sediment transport considered?  Yes  No If yes, then fill out Section F (Sediment Transport) of Form 3. If No, then attach your explanation for why sediment transport was not considered.

**B. HYDRAULICS**

1. Reach to be Revised

	Description	Cross Section	Water-Surface Elevations (ft.)	
			Effective	Proposed/Revised
Downstream Limit	Confluence with the Santa Cruz River	St#93.58		
Upstream Limit	N. Via Sinuosa Rd..	St# 6337.23		

2. Hydraulic Method/Model Used

HEC-RAS

## B. HYDRAULICS (CONTINUED)

### 3. Pre-Submittal Review of Hydraulic Models

DHS-FEMA has developed two review programs, CHECK-2 and CHECK-RAS, to aid in the review of HEC-2 and HEC-RAS hydraulic models, respectively. These review programs may help verify that the hydraulic estimates and assumptions in the model data are in accordance with NFIP requirements, and that the data are comparable with the assumptions and limitations of HEC-2/HEC-RAS. CHECK-2 and CHECK-RAS identify areas of potential error or concern. **These tools do not replace engineering judgment.** CHECK-2 and CHECK-RAS can be downloaded from [http://www.fema.gov/plan/prevent/fhm/frm\\_soft.shtm](http://www.fema.gov/plan/prevent/fhm/frm_soft.shtm). We recommend that you review your HEC-2 and HEC-RAS models with CHECK-2 and CHECK-RAS. Review of your submittal and resolution of valid modeling discrepancies may result in reduced review time.

### 4. Models Submitted

	<u>Natural Run</u>	<u>Floodway Run</u>	<u>Datum</u>
Duplicate Effective Model*	File Name: N/A    Plan Name: N/A	File Name: N/A    Plan Name:	
Corrected Effective Model*	File Name: DelCerroWash    Plan Name: Plan 1	File Name:    Plan Name:	NAVD 88
Existing or Pre-Project Conditions Model	File Name: N/A    Plan Name:	File Name:    Plan Name:	_____
Revised or Post-Project Conditions Model	File Name: N/A    Plan Name:	File Name:    Plan Name:	_____
Other - (attach description)	File Name: N/A    Plan Name:	File Name:    Plan Name:	_____

\* For details, refer to the corresponding section of the instructions.

Digital Models Submitted? (Required)

## C. MAPPING REQUIREMENTS

A **certified topographic map** must be submitted showing the following information (where applicable): the boundaries of the effective, existing, and proposed conditions 1%-annual-chance floodplain (for approximate Zone A revisions) or the boundaries of the 1%- and 0.2%-annual-chance floodplains and regulatory floodway (for detailed Zone AE, AO, and AH revisions); location and alignment of all cross sections with stationing control indicated; stream, road, and other alignments (e.g., dams, levees, etc.); current community easements and boundaries; boundaries of the requester's property; certification of a registered professional engineer registered in the subject State; location and description of reference marks; and the referenced vertical datum (NGVD, NAVD, etc.).

Digital Mapping (GIS/CADD) Data Submitted

Note that the boundaries of the existing or proposed conditions floodplains and regulatory floodway to be shown on the revised FIRM and/or FBFM must tie-in with the effective floodplain and regulatory floodway boundaries. Please attach a **copy of the effective FIRM and/or FBFM**, annotated to show the boundaries of the revised 1%- and 0.2%-annual-chance floodplains and regulatory floodway that tie-in with the boundaries of the effective 1%- and 0.2%-annual-chance floodplain and regulatory floodway at the upstream and downstream limits of the area of revision.

Annotated FIRM and/or FBFM (Required)

## D. COMMON REGULATORY REQUIREMENTS\*

1. For LOMR/CLOMR requests, do Base Flood Elevations (BFEs) increase?  Yes  No
  - a. For CLOMR requests, if either of the following is true, please submit **evidence of compliance with Section 65.12 of the NFIP regulations**:
    - The proposed project encroaches upon a regulatory floodway and would result in increases above 0.00 foot.
    - The proposed project encroaches upon a SFHA with or without BFEs established and would result in increases above 1.00 foot.
  - b. For LOMR requests, does this request require property owner notification and acceptance of BFE increases?  Yes  No  
 If Yes, please attach **proof of property owner notification and acceptance (if available)**. Elements of and examples of property owner notification can be found in the MT-2 Form 2 Instructions.
  
2. Does the request involve the placement or proposed placement of fill?  Yes  No  
 If Yes, the community must be able to certify that the area to be removed from the special flood hazard area, to include any structures or proposed structures, meets all of the standards of the local floodplain ordinances, and is reasonably safe from flooding in accordance with the NFIP regulations set forth at 44 CFR 60.3(a)(3), 65.5(a)(4), and 65.6(a)(14). Please see the MT-2 instructions for more information.
  
3. For LOMR requests, is the regulatory floodway being revised?  Yes  No  
 If Yes, attach **evidence of regulatory floodway revision notification**. As per Paragraph 65.7(b)(1) of the NFIP Regulations, notification is required for requests involving revisions to the regulatory floodway. (Not required for revisions to approximate 1%-annual-chance floodplains [studied Zone A designation] unless a regulatory floodway is being added. Elements and examples of regulatory floodway revision notification can be found in the MT-2 Form 2 Instructions.)
  
4. For LOMR/CLOMR requests, does this request have the potential to impact an endangered species?  Yes  No  
 If Yes, please submit documentation to the community to show that you have complied with Sections 9 and 10 of the Endangered Species Act (ESA). Section 9 of the ESA prohibits anyone from "taking" or harming an endangered species. If an action might harm an endangered species, a permit is required from U.S. Fish and Wildlife Service or National Marine Fisheries Service under Section 10 of the ESA.  
  
 For actions authorized, funded, or being carried out by Federal or State agencies, please submit documentation from the agency showing its compliance with Section 7(a)(2) of the ESA.

\* Not inclusive of all applicable regulatory requirements. For details, see 44 CFR parts 60 and 65.



## **Appendix C: Survey Field Notes**

**Evan Canfield**

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**From:** Kenneth Maits  
**Sent:** Monday, May 03, 2010 12:20 PM  
**To:** Evan Canfield  
**Subject:** FW: PAG 2008 Orthos/Lidar

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**From:** Curtis, Edward [mailto:Edward.Curtis@dhs.gov]  
**Sent:** Tuesday, November 10, 2009 2:44 PM  
**To:** Manny M. Rosas  
**Cc:** Terry Hendricks; Lucero, Andrew; Caldwell, Jason; Akl, Pascal  
**Subject:** RE: PAG 2008 Orthos/Lidar

Mr. Rosas –

I apologize for the delay in responding to you regarding the Sanborn LiDAR report. Pascal Akl of Michael Baker, Jr. reviewed the updated July 2009 report on behalf of FEMA and advised me that all of the concerns raised in his May 18, 2009 memorandum titled "Pima County, CA [sic] Sanborn LiDAR Report Items" were addressed in the updated report except the comment that the original report lacked a sufficient number of checkpoints in urban areas and dense vegetation areas. No additional checkpoints were surveyed in such arease to permit analysis of data accuracy in these land cover categories. However, in the data voids analysis section of the updated report (p. 16), Sanborn states the following: "*Specific areas, dense vegetation or undergrowth near small streams, for example, prevents the LiDAR pulses to fully penetrate to the true ground surface. Thus, for mapping products such as floodplain or contour mapping, LiDAR data must often be manually supplemented with breaklines and mass-points to accurately model the terrain surface.*" As long as the data is used with caution and supplemented with additional ground survey data where necessary in accordance with this statement, I am satisfied that the terrain data meets FEMA standards for use in detailed flood studies.

Please contact me if you have any questions regarding our review and comments.

**Ed Curtis, P.E., CFM**  
**Risk Analysis Branch**  
**FEMA Region IX**  
**(510) 627-7207 - office**  
**(510) 295-5249 - mobile**

---

**From:** Manny M. Rosas [mailto:MRosas@pagnet.org]  
**Sent:** Tuesday, November 10, 2009 7:29 AM  
**To:** 'Lucero, Andrew'; 'Caldwell, Jason'  
**Cc:** 'Terry Hendricks'; Curtis, Edward  
**Subject:** PAG 2008 Orthos/Lidar

Hi Andy,  
I resent Sanborn's Version 3 document produced in July 2009 and yet to receive any comments from FEMA, Pima County and Michael Baker Inc. therefore please proceed with direct communications with Michael Baker Inc (Pascal Akl) to resolve all issues regarding the FEMA guidelines

Thank You  
Manny

5/6/2010

*Manny M. Rosas Jr.*  
*GIS Administrator*



**Pima Association of Governments**

177 N Church Ave. Suite 405  
Tucson, Arizona 85701

520-792-1093 (tel)  
520-620-6981 (fax)

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112279	SILVERBELL RD, GRANT RD. TO INA RD. (C.O.T.) (REF. JOB NO. 15794 IN HOR F.B. 201)	ALL
	COT FB # 2255	
<p>I HEREBY CERTIFY THAT THE SURVEY CONTAINED HEREIN WAS CONDUCTED UNDER MY DIRECT SUPERVISION BETWEEN MAY, 2009 AND MAY, 2010. I FURTHER CERTIFY THAT THIS SURVEY WAS CONDUCTED AS A CONTROL AND DESIGN SURVEY FOR SILVERBELL RD. FROM GRANT RD. TO INA RD. THE INFORMATION CONTAINED HEREIN IS TRUE, ACCURATE AND SUFFICIENT TO RETRACE THIS SURVEY. THIS SURVEY MEETS OR EXCEEDS THE MINIMUM STANDARDS FOR SURVEYS IN ARIZONA.</p>		





**PIMA COUNTY  
REGIONAL FLOOD CONTROL DISTRICT**  
97 EAST CONGRESS STREET, THIRD FLOOR  
TUCSON, ARIZONA 85701-1797

**SUZANNE SHIELDS, P.E.  
DIRECTOR**

**(520) 243-1800  
FAX (520) 243-1821**

January 2, 2009

Craig S. Kennedy, CFM, Program Specialist  
Engineering Management Branch  
Mitigation Directorate  
FEMA  
500C Street SW  
Washington, DC 20472

**Re: Re: Acceptability of LiDAR**

Dear Mr. Kennedy:

The Pima Association of Governments (PAG) has contracted with Sanborn to generate ortho rectified aerial photography and LiDAR. Figure 1 shows the extent of the LiDAR coverage for Pima County and the FIRM Special Flood Hazard Areas. The next version of Flood Insurance Rate Maps for Pima County will be converted to the North American Vertical Datum of 1988 (NAVD88). Attached to this letter you will find a draft letter from Sanborn indicating the vertical accuracy of the LiDAR meets FEMA's Map Modernization requirements.

The Pima County Regional Flood Control District (District) requests that the documentation in the draft letter be examined by FEMA to verify the LiDAR and topography created from this data would meet FEMA's vertical requirements for mapping to the NAVD88 Datum. If acceptable, the District will request PAG to have Sanborn seal the documentation so that it may be used in FEMA re-mapping processes. The District understands the digital maps generated from the LiDAR would need to be re-projected to FEMA's UTM coordinate system for LOMR applications.

Please call me at 520-243-1800, should you have any questions with this request.

Sincerely,

A handwritten signature in blue ink, appearing to read "Terry Hendricks".





R. "Terry" Hendricks, CFM, Chief Hydrologist  
Planning and Development Division

RTH/cd

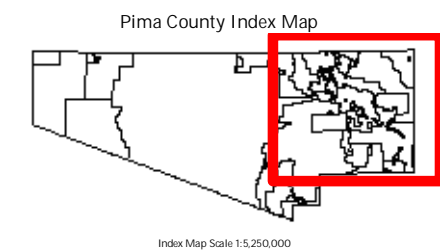
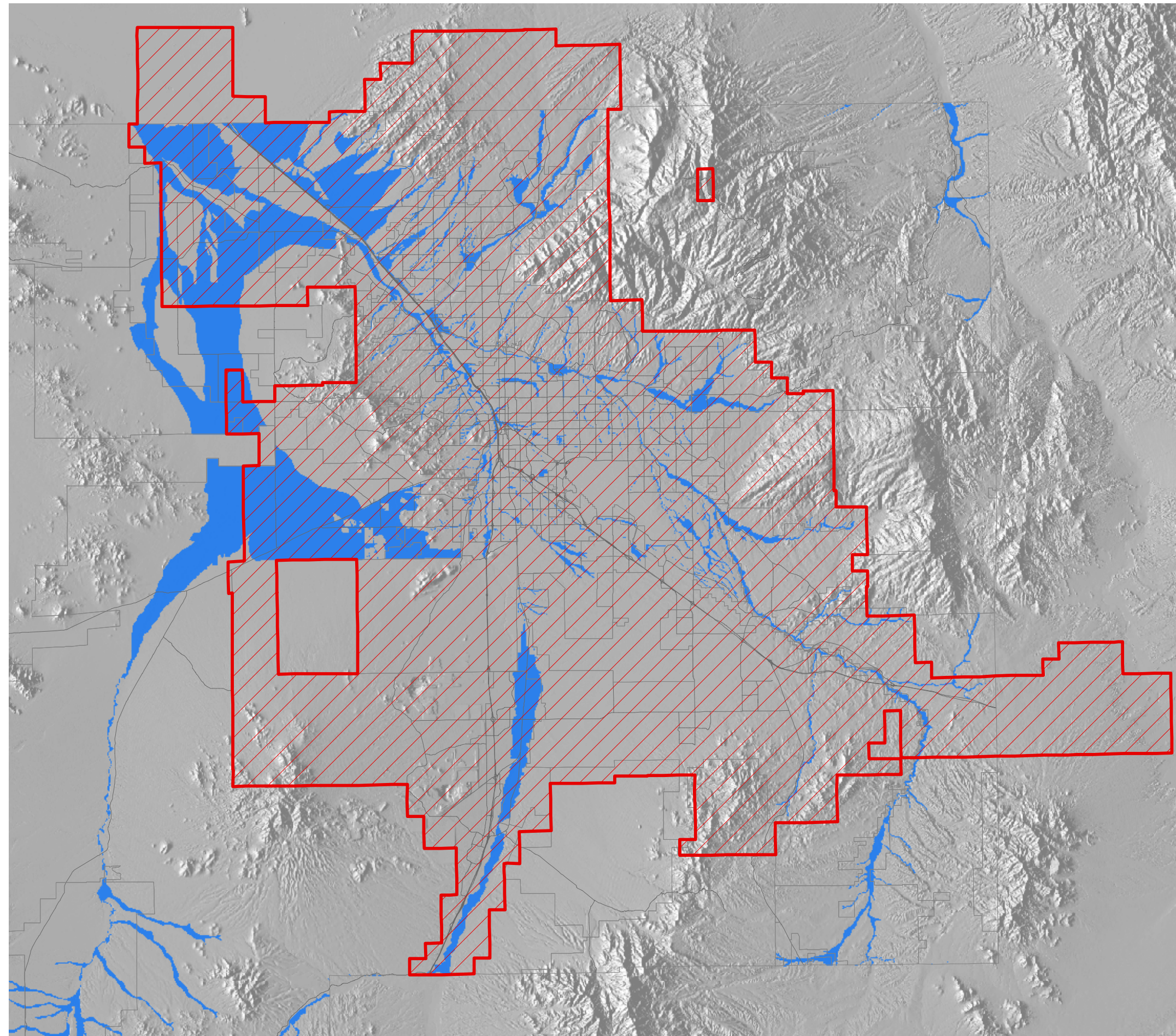
Cc: Steve Whitney, GIS Manager, Pima County Department of Transportation  
Kenneth Maits, Senior GIS Analyst, PC Regional Flood Control District  
Manny M. Rosas, GIS Administrator, Pima Association of Governments.

Enclosures

# Exhibit 1: 2008 LiDAR Coverage and FEMA Special Flood Hazard Areas

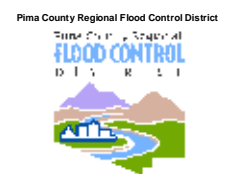

-  2008 LiDAR Coverage
-  FEMA Floodplains
-  Major Streets
-  Jurisdiction Lines

Not Shown: Western Pima County, including Ajo and LiDAR coverage on Tohono O'dham Nation.



The information depicted on this display is the result of digital analyses performed on a variety of databases provided and maintained by several governmental agencies. The accuracy of the information presented is limited to the collective accuracy of these databases on the date of the analysis. The Pima County Regional Flood Control Department makes no claims regarding the accuracy of the information depicted herein.

This product is subject to the Department of Transportation Technical Services Division's Use Restriction Agreement.

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Missouri

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Middle East

Mumbai  
India

30 December 2008

Manny Rosas, GIS Administrator  
Pima Association of Governments  
177 N. Church Ave.  
Suite 405  
Tucson, AZ 85701

Re: FEMA Results for the PAG 2008 Ortho Project (Contract – 08-5951-01)

Dear Mr. Rosas,

Attached you will find the results of the FEMA checkpoints for PAG 2008 LiDAR data. Sanborn's contracted Arizona State Registered Land-Surveyor, Greg Thompson, performed a review of the report and is in agreement with the results.

### **Background**

To ensure the accuracy of the PAG 2008 LiDAR data, Sanborn was contracted to implement a project plan that included the integration of FEMA checkpoints as part of the QA/QC process. To support this initiative, Sanborn collected 69 checkpoints as part of the control survey effort. This meets the minimum standards for vertical accuracy testing and reporting as defined in FEMA's map modernization requirements. FEMA recommends 20 checkpoints in each of the major land cover categories representative of floodplains being mapped; this normally requires a minimum of 60 checkpoints for at least three land cover categories. The three categories surveyed were:

1. Bare Earth and Low Grass
2. High Grass, Weeds, and Crops
3. Brush lands/low trees

Field data was acquired using GPS equipment and static surveying methods. Sanborn team surveyed all checkpoint following the procedures in NOAA Technical Memorandum NOS NGS-58, "Guidelines for Establishing GPS-Derived Ellipsoid Heights (Standards: 2 cm and 5 cm)" and use NGS' latest Geoid Model to compute NAVD88 orthometric heights accurate to 5-cm at the 95% confidence level. (The x and y coordinates of checkpoints will be accurate to 2-cm at the 95% confidence level.)

Final adjusted results were adjusted to Arizona State Plane Coordinates, Central Zone NAD83-92 (HARN), NAVD88, in units of International Foot.

### **Testing Methodology**

As stated in the Guidelines and Specifications for Flood Hazard Mapping Partners (April 2003), Section A.87.6.1, "The Root Mean Square Error (RMSE) is the square root of the average of the set of squared differences between dataset coordinate values and coordinate values from an independent source of higher accuracy for identical points... TINs (and DEMs derived therefrom) should normally have a maximum RMSE of 18.5 centimeters, equivalent to

2-foot contours, in flat terrain. The following are the results from the PAG 2008 Ortho Program.

**Bare Earth:**

Number	Easting	Northing	Known Z	Laser Z	Dz -----
1	1001837.162	410093.611	2587.031	2587.530	+0.499
7	999345.782	449442.944	2441.401	2441.640	+0.239
20	933721.166	412981.849	2439.091	2439.310	+0.219
17	933650.558	412993.658	2438.451	2438.660	+0.209
15	897369.501	552863.803	1937.257	1937.430	+0.173
3	1159921.689	349431.234	4294.062	4294.210	+0.148
8	1005806.086	434836.185	2509.196	2509.340	+0.144
9	1000927.946	448200.185	2446.309	2446.400	+0.091
13	994444.372	503631.914	2655.984	2656.070	+0.086
11	994513.846	503595.055	2658.012	2658.090	+0.078
18	955798.751	425094.504	2540.814	2540.880	+0.066
2	906521.919	540616.247	1946.585	1946.640	+0.055
12	987338.200	503575.338	2542.972	2542.930	-0.042
14	965580.705	519074.819	2667.182	2667.120	-0.062
21	955893.647	425085.496	2541.302	2541.240	-0.062
5	1021871.892	457772.536	2472.149	2472.080	-0.069
6	1033139.499	445741.877	2610.656	2610.580	-0.076
16	939704.593	416728.203	2432.726	2432.640	-0.086
10	985754.835	454784.703	2313.130	2312.980	-0.150
19	939508.793	416651.451	2432.802	2432.630	-0.172
22	1027485.930	416573.872	2740.833	2740.650	-0.183
4	1156429.917	365109.827	4055.003	outside	*

Average dz	+0.053
Minimum dz	-0.183
Maximum dz	+0.499
Average magnitude	0.139
Root mean square	<b>0.171 (foot)</b>
Std deviation	0.166

**Medium Vegetation:**

Number	Easting	Northing	Known Z	Laser Z	Dz -----
1	1187028.525	351518.925	4080.561	4080.540	-0.021
2	1117108.620	363592.785	3587.077	3586.940	-0.137
3	1073972.909	383419.761	3240.515	3240.590	+0.075
4	1010832.502	410849.441	2643.786	2643.990	+0.204
5	1005445.314	419156.617	2579.495	2579.880	+0.385
6	1045092.088	435136.157	2724.009	2724.050	+0.041



7	1069748.640	446273.392	2760.125	2760.050	-0.075
8	1033371.126	464364.855	2518.606	2518.310	-0.296
9	1054207.161	418935.174	2886.854	2886.950	+0.096
10	955203.805	487660.945	2178.734	2179.130	+0.396
11	997532.713	434803.804	2460.164	2460.250	+0.086
12	979509.921	492673.940	2417.792	2418.190	+0.398
13	995655.491	465569.990	2344.777	2344.690	-0.087
14	997550.935	482620.376	2598.648	2598.550	-0.098
15	1001334.429	474026.061	2493.153	2493.240	+0.087
16	990196.690	487542.780	2546.083	outside	*
17	990519.334	490352.801	2559.039	2559.110	+0.071
18	998219.739	493708.248	2936.804	2937.080	+0.276
19	996795.607	504234.682	2727.497	2727.370	-0.127
20	988245.902	501104.027	2560.988	2560.870	-0.118
21	985960.009	501595.086	2553.169	2553.130	-0.039
22	997446.853	506178.000	2694.613	2695.140	+0.527
23	987398.768	503506.302	2546.335	2546.410	+0.075
24	985971.797	501493.493	2552.516	2552.570	+0.054
25	997540.656	506124.929	2707.864	2708.230	+0.366
26	991206.370	506306.455	2518.406	2518.250	-0.156
27	978945.698	519233.465	2782.405	2782.330	-0.075
28	978935.642	519272.398	2784.006	2784.080	+0.074
29	965555.375	519044.382	2666.260	2666.260	+0.000
30	897298.425	552978.606	1937.352	1937.730	+0.378
31	910066.011	514280.384	2003.658	2003.840	+0.182

Average dz	+0.085
Minimum dz	-0.296
Maximum dz	+0.527
Average magnitude	0.167
Root mean square	<b>0.217 (foot)</b>
Std deviation	0.203

**High Vegetation:**

Number	Easting	Northing	Known Z	Laser Z	Dz
1	1041505.790	408998.331	2868.881	2869.410	+0.529
4	1007421.616	441240.211	2501.880	2502.270	+0.390
9	988302.547	500937.045	2557.959	2558.170	+0.211
10	993323.041	504876.742	2616.818	2616.900	+0.082
3	944799.536	483176.205	2406.404	2406.480	+0.076
6	993338.640	505132.410	2616.096	2616.130	+0.034
13	995168.385	519848.931	2773.051	2773.040	-0.011
8	996811.199	504124.980	2733.504	2733.470	-0.034
14	995094.857	519807.072	2771.624	2771.590	-0.034
5	995053.089	492295.493	2741.552	2741.500	-0.052
7	986911.443	504348.439	2463.848	2463.780	-0.068

11	986965.447	504425.310	2458.159	2458.090	-0.069
12	993296.411	506167.522	2598.730	2598.640	-0.090
16	919968.908	521623.590	2003.520	2003.130	-0.390
15	909979.986	514314.158	2004.186	2003.740	-0.446
2	988498.629	488163.006	2506.243	outside	*

Average dz	+0.009
Minimum dz	-0.446
Maximum dz	+0.529
Average magnitude	0.168
Root mean square	<b>0.240 (foot)</b>
Std deviation	0.248

Sanborn concludes that the overall RMSE of the LiDAR data is within PAG 2008 Ortho project requirements, as it meets the +/- 15.0 cm (0.492 foot) RMSE at 95% confidence for all three categories. RMSE is an indicator of overall accuracy of the product and is not used for individual point accuracy.

Please contact me at (719) 593-0093 extension 5645 or Jamie Young (General Manager – ext. 5602) if you have any question regarding the report.

Sincerely,

Andrew Lucero  
Sanborn  
Senior Project Manager

## **Appendix D: Hydrology**

Supporting documentation is included as digital data on the CD.

## **Appendix E: Hydraulics**

Supporting documentation is included as digital data on the CD.

## **Appendix F: Erosion and Sediment Transport Analysis Supporting Documentation**

None